

WORKSHOP ON PENETRATION TESTING AND OTHER GEOMECHANICAL ISSUES

Pisa 14 June 2016 – ROOM F8

CPTU IN UNUSUAL SOILS

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UNIVERSITA' DI PISA

LECTURE OUTLINE

- **DEFINITION OF UNUSUAL SOILS**
- **CHARACTERIZATION (IDENTIFICATION) OF PARTIALLY SATURATED FINE- GRAINED SOILS**
- **UNDERCONSOLIDATED SILTY SOILS**



UNUSUAL SOILS

- **LAYERED;**
- **PARTIALLY SATURATED;**
- **UNDERCONSOLIDATED;**
- **INTERMEDIATE PERMEABILITY;**
- **COMPACTED SOILS**



EQUIPMENT AT UNIPI



L'ORDINE DEGLI INGEGNERI
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CPTU EQUIPMENTS



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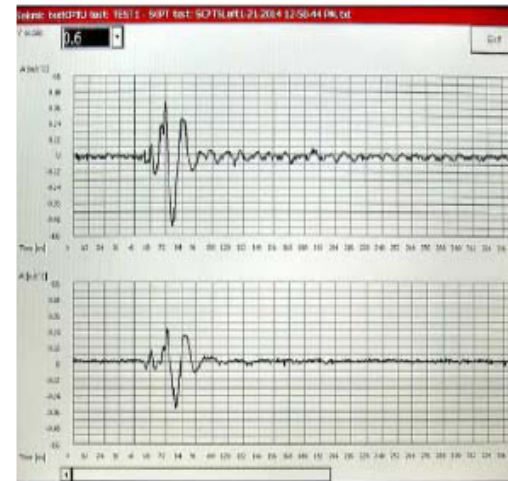
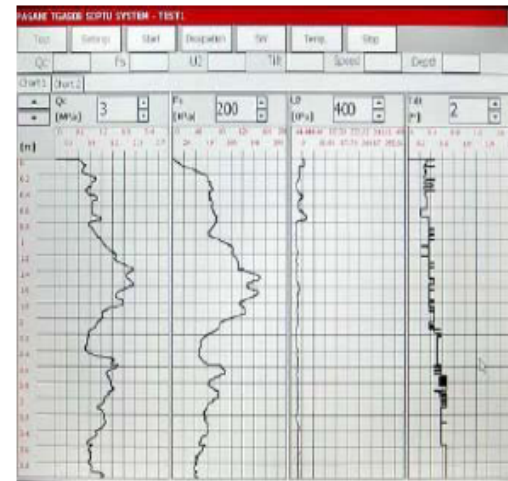
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SCPTU EQUIPMENT



- 2 TX accelerometers at 0.5 m distance
- Connector
- Source



Screenshot of CPTu and SCPTu test

SCPTU - ROTOSEIS

Equipment: Seismic piezocone (source 2: Rotoseis) Mayne 2010

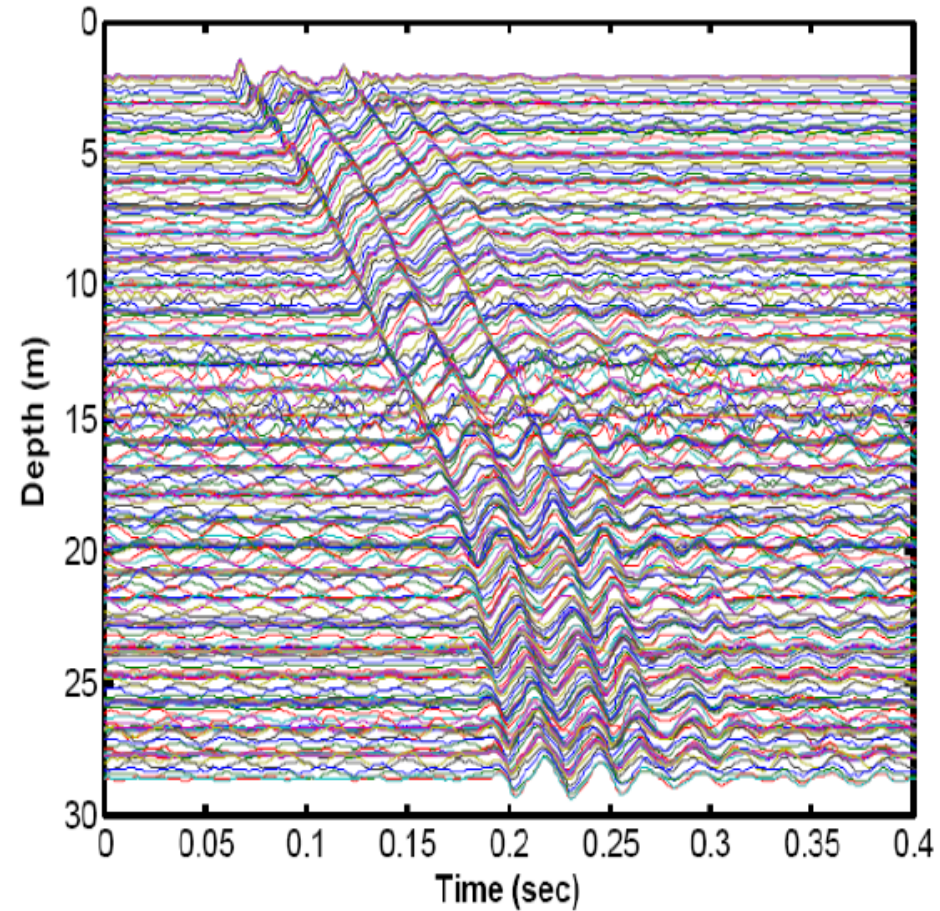
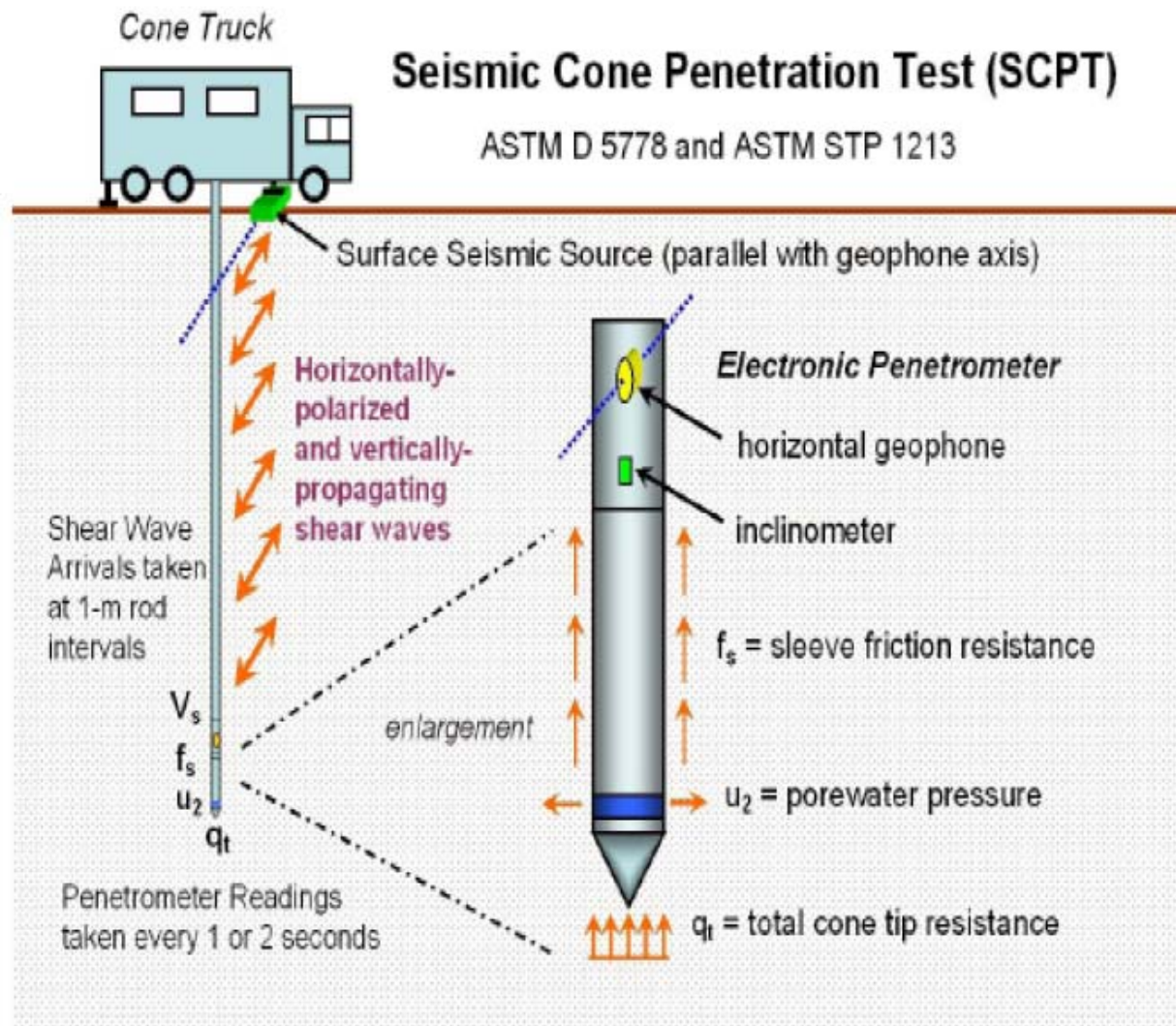


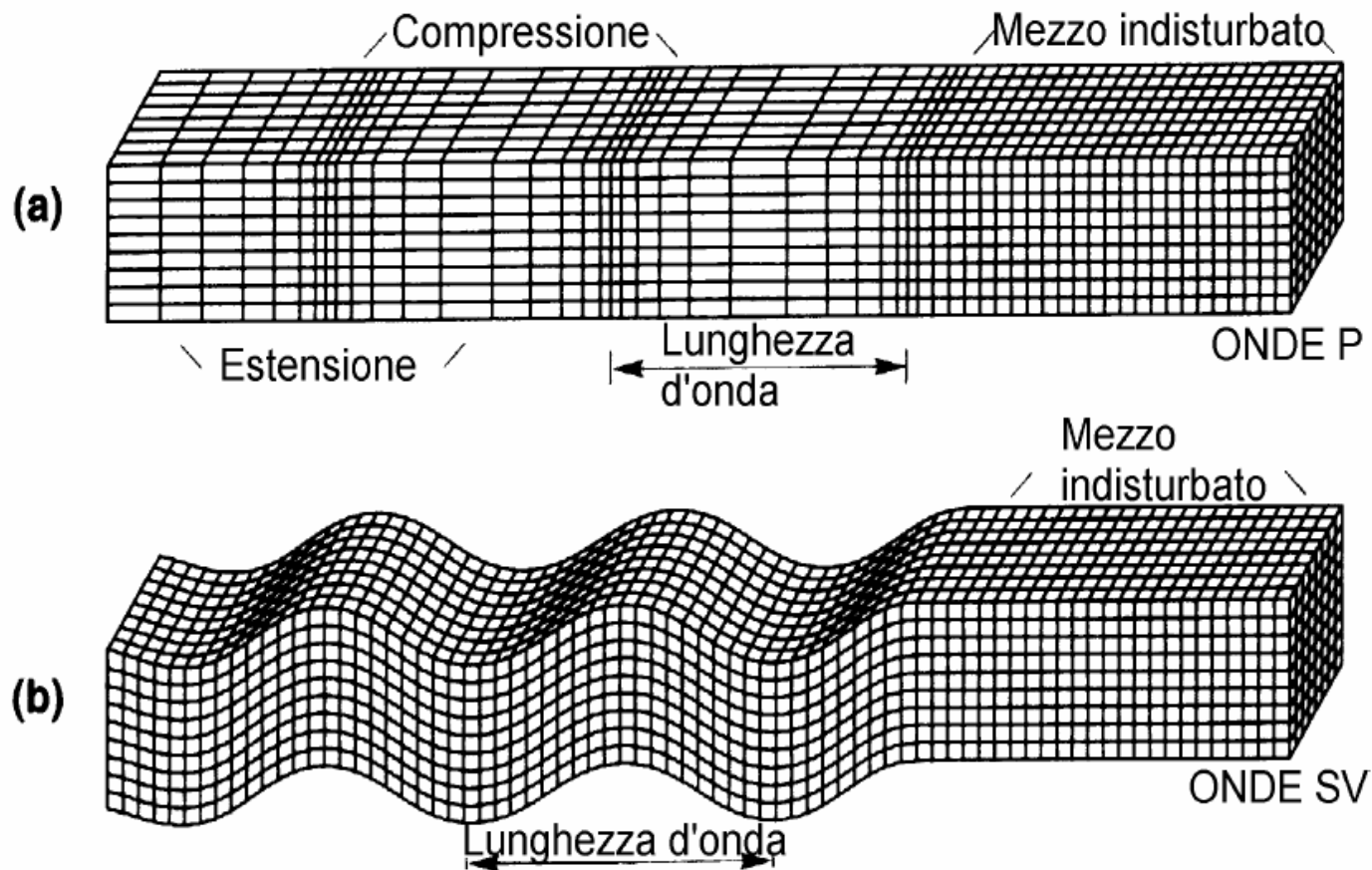
Figure 25. Summary paste-up of wavelets from continuous seismic cone testing in Charleston, SC (McGillivray & Mayne, 2008).

SCPTU INTERPRETATION



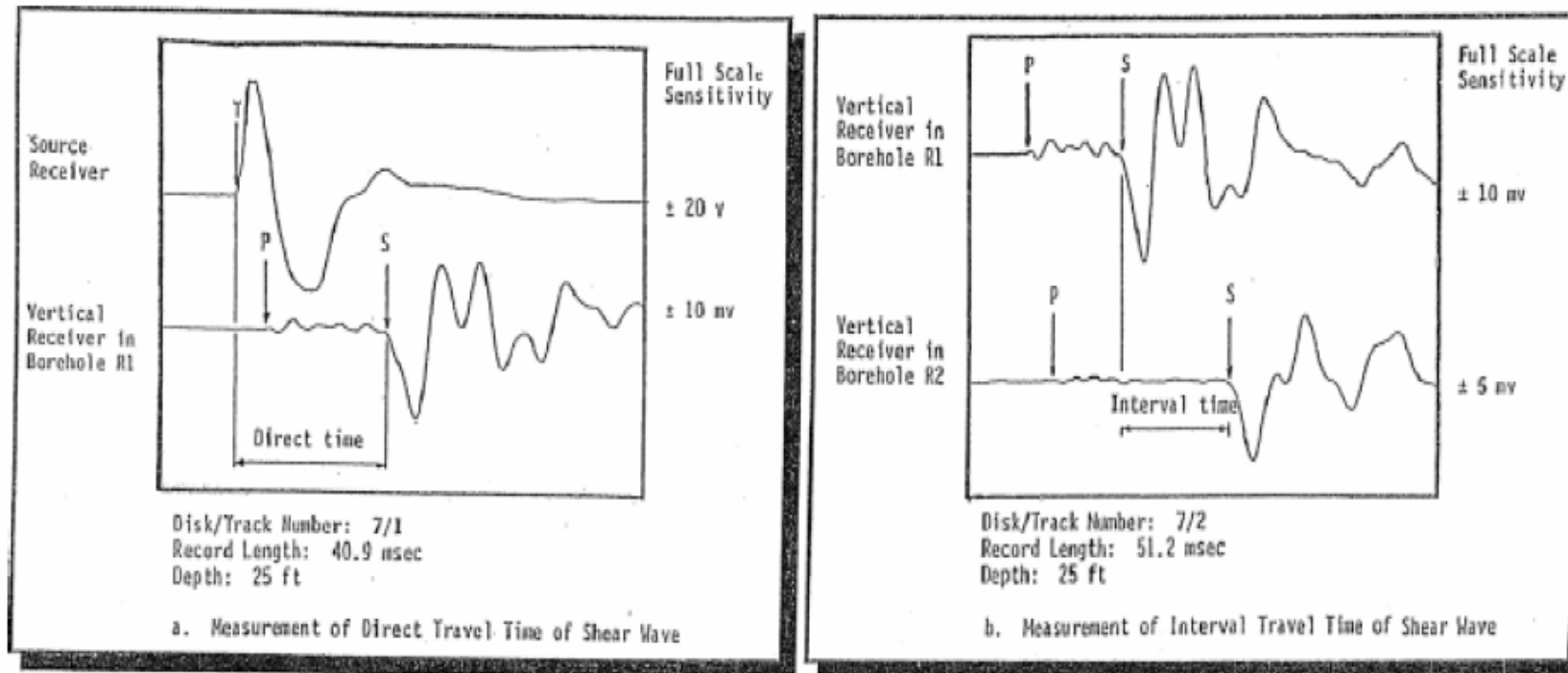
SCPTU INTERPRETATION

Body waves (Bolt 1988)



ARRIVAL TIME PICKUP

Test interpretation (one or two – Stokoe 1992)



HOMOGENEOUS – ELASTIC – ISOTROPIC MEDIUM

PROPAGATION VELOCITIES

$$V_s = d/t_s \quad V_p = d/t_p \quad V_R = f(\omega) = \lambda f$$

ELASTIC PARAMETERS

$$G_0 = \rho V_s^2 \quad M_0 = \rho V_p^2 \quad V_R / V_s \cong (0.862 + 1.14\nu) / (1 + \nu)$$

$$\nu = (V_p^2 - 2V_s^2) / 2 (V_p^2 - V_s^2)$$

$$\gamma = \frac{u_s}{V_s}$$



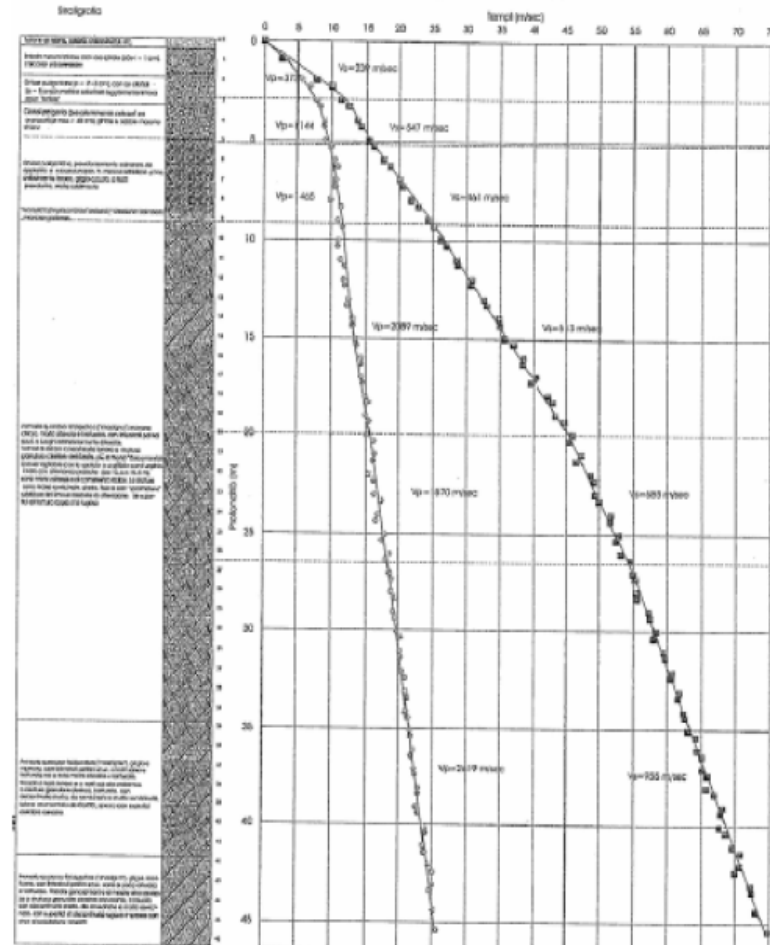
INTERPRETATION METHODS

- ARRIVAL TIME PICKUP
 - PSEUDO – INTERVAL
 - TRUE INTERVAL
- CROSS CORRELATION



CORRECTED-TIME VS. DEPTH

Pseudo - interval



USE OF V_p IN PARTIALLY SATURATED SOILS



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ANY REASON TO MEASURE V_p ?

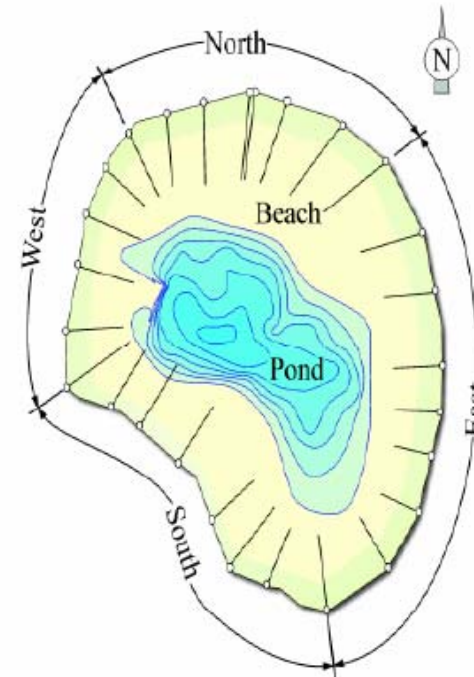
CPTU and V_p measurements



Figure 1. Location of the Zelazny Most depository.



Maximum dam height:	63 m
Total volume stored:	$527 \times 10^6 \text{ m}^3$
Storage rate:	$\cong 17.5 \times 10^6 \text{ m}^3/\text{annum}$
Area covered:	12.4 km^2
Total length of dam:	14.3 km



Dams Height, m

North:	39
West:	49
South:	34
East:	63

Crest Elev., m asl

North:	177.5
West:	177.5
South:	177.0
East:	177.5

JAMIOLKOWSKI 2013 – RANKINE LECTURE

U2 from CPTU

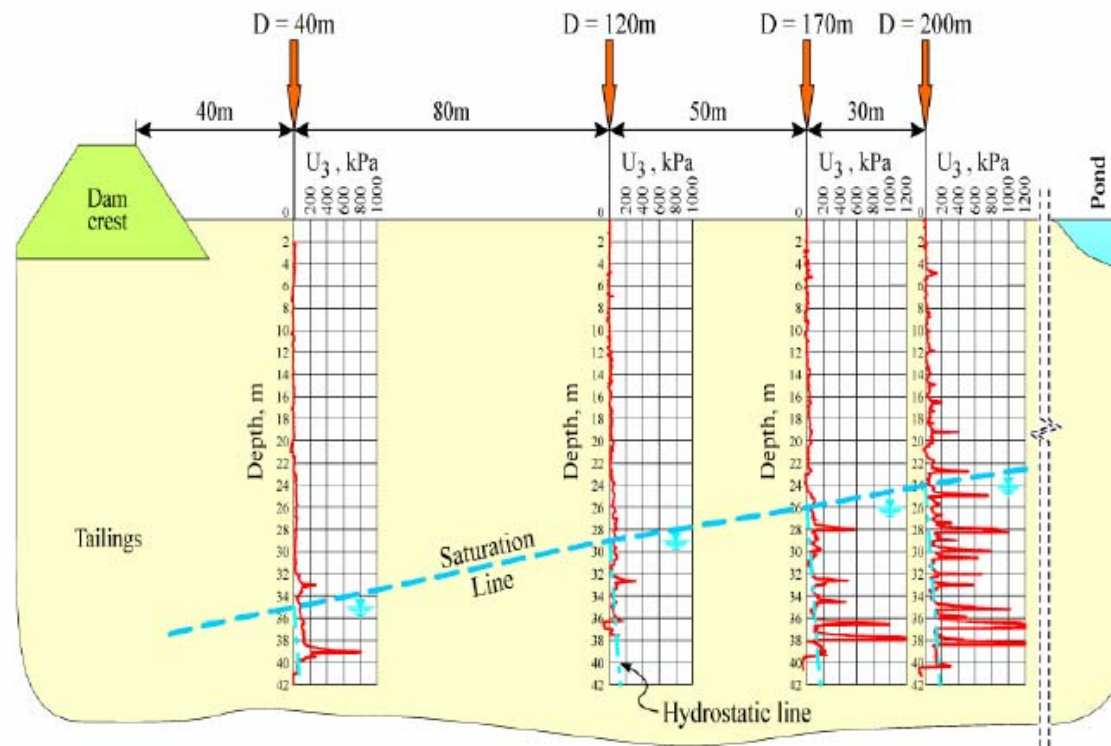


Figure 9. West Dam - Penetration pore pressure from CPTU.



JAMIOLKOWSKI 2013 – RANKINE LECTURE

V_p from down hole measurements

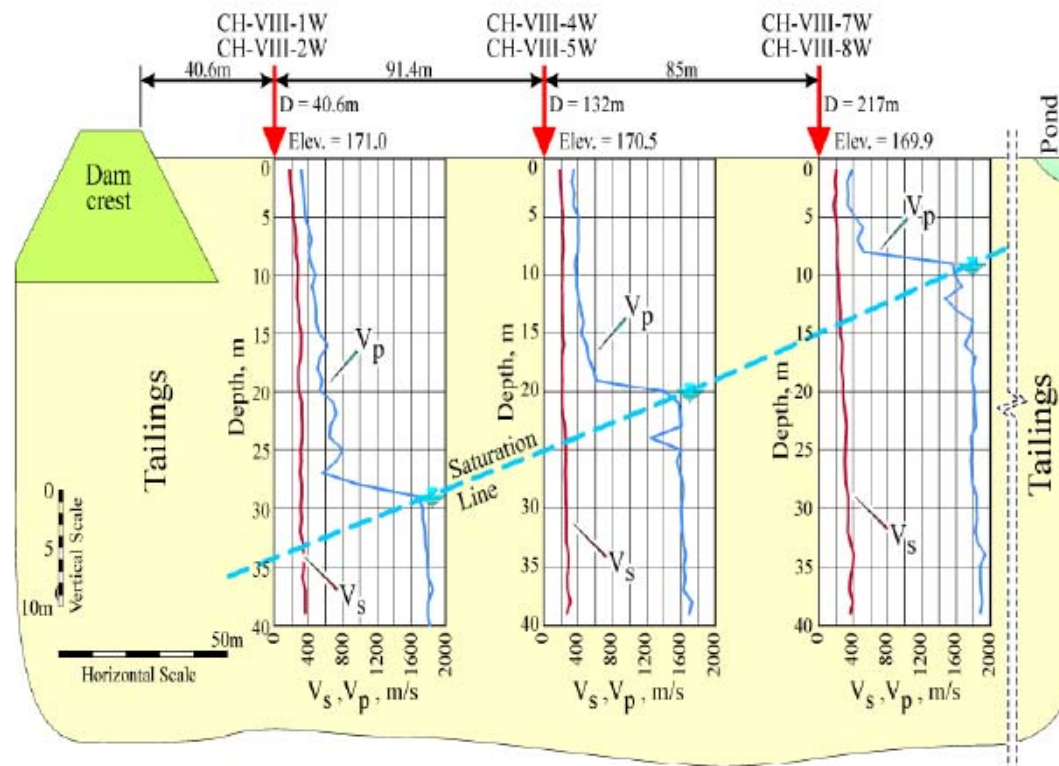


Figure 12. West Dam - Location of saturation line from V_p measurements.



CC TESTS ON TICINO SAND 1

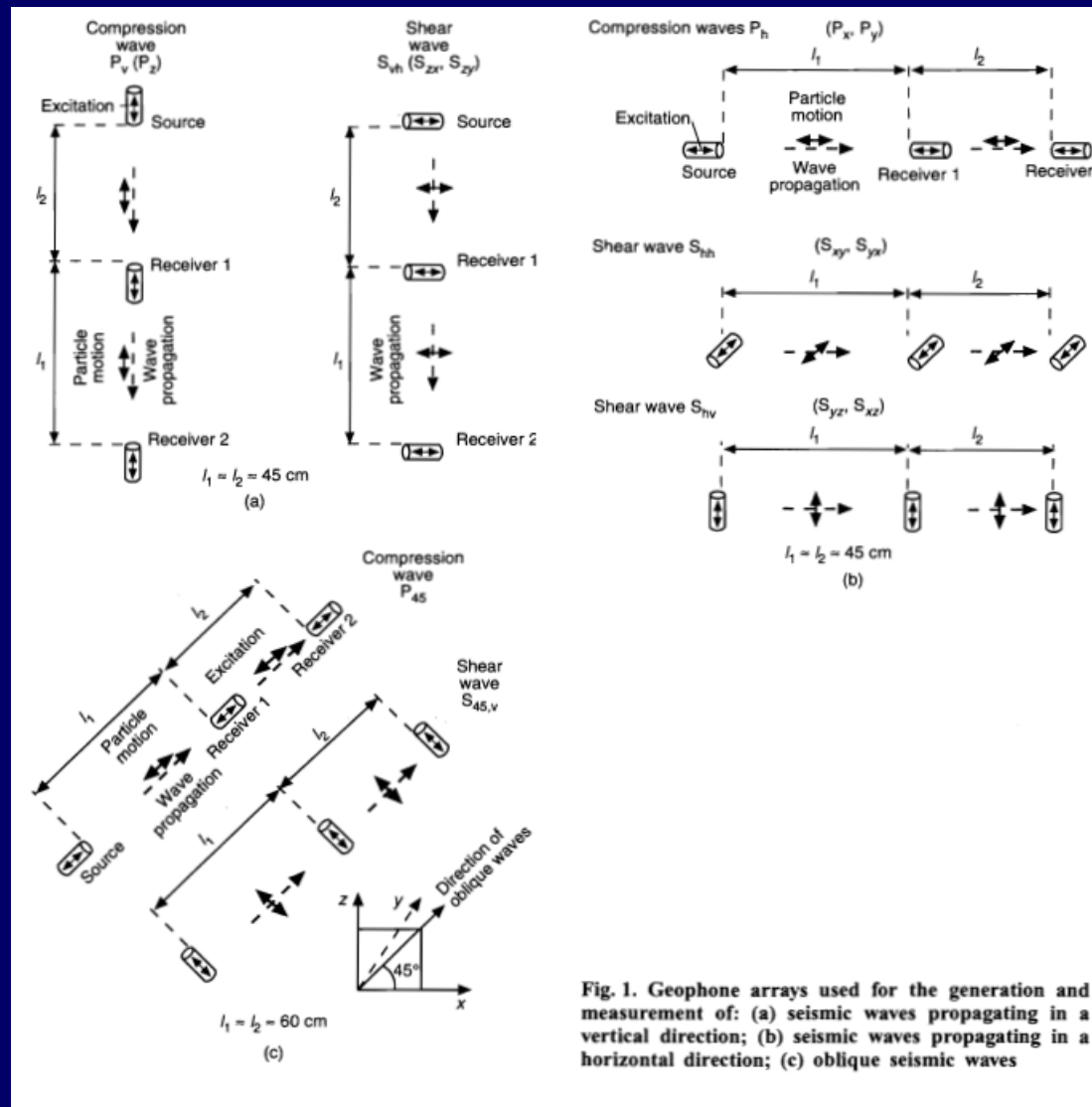


Fig. 1. Geophone arrays used for the generation and measurement of: (a) seismic waves propagating in a vertical direction; (b) seismic waves propagating in a horizontal direction; (c) oblique seismic waves



CC TESTS ON TICINO SAND 2

$$V_s = C_s \sqrt{[F(e)(\sigma_a')^{na}(\sigma_b')^{nb}(\sigma_c')^{nc}]} \quad (3)$$

$$V_p = C_p \sqrt{[F(e)(\sigma_a')^{na}(\sigma_b')^{nb}(\sigma_c')^{nc}]} \quad (4)$$

$$G_0 = C_G F(e) p_a^{(1-2na-2nb-2nc)} (\sigma_a')^{2na} \\ \times (\sigma_b')^{2nb} (\sigma_c')^{2nc} \quad (5)$$

$$M_0 = C_M F(e) p_a^{(1-2na-2nb-2nc)} (\sigma_a')^{2na} \\ \times (\sigma_b')^{2nb} (\sigma_c')^{2nc} \quad (6)$$



INHERENT ANISOTROPY

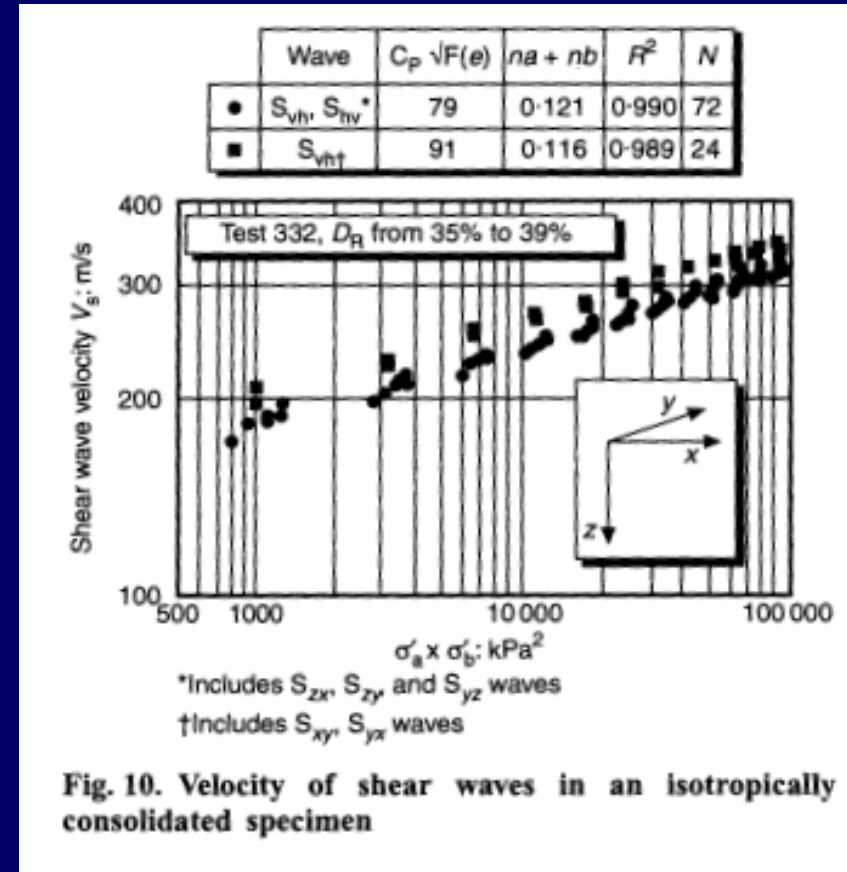
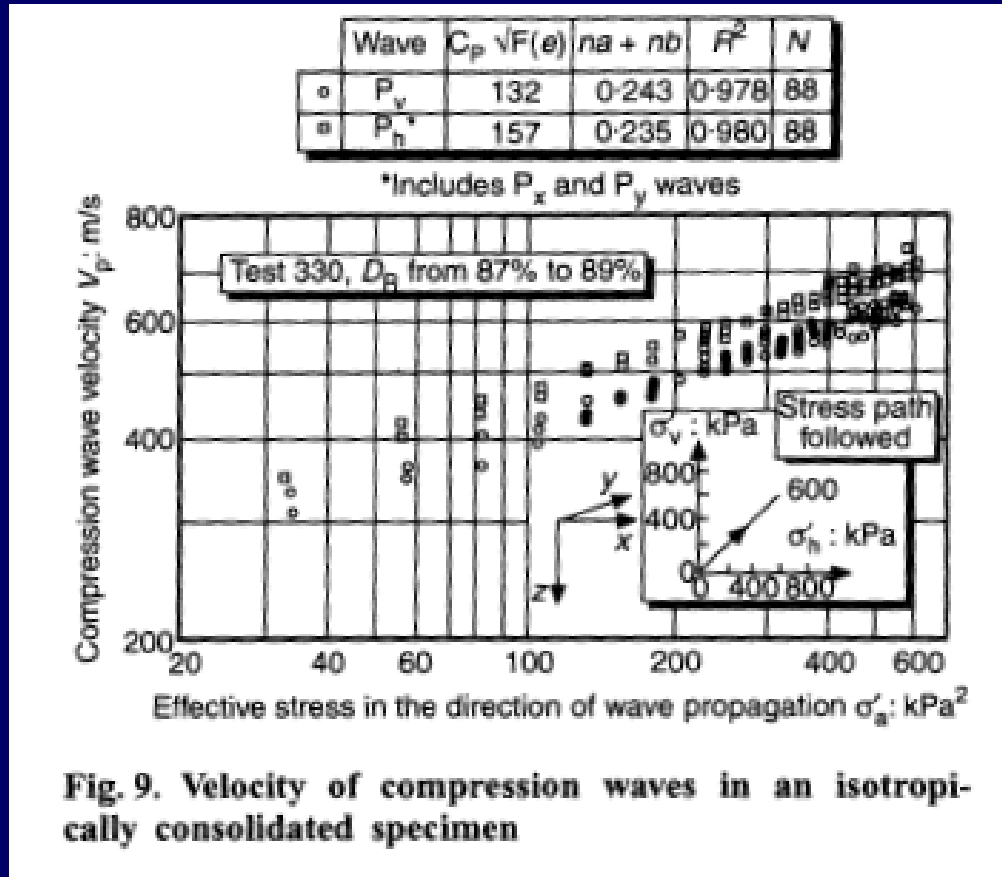
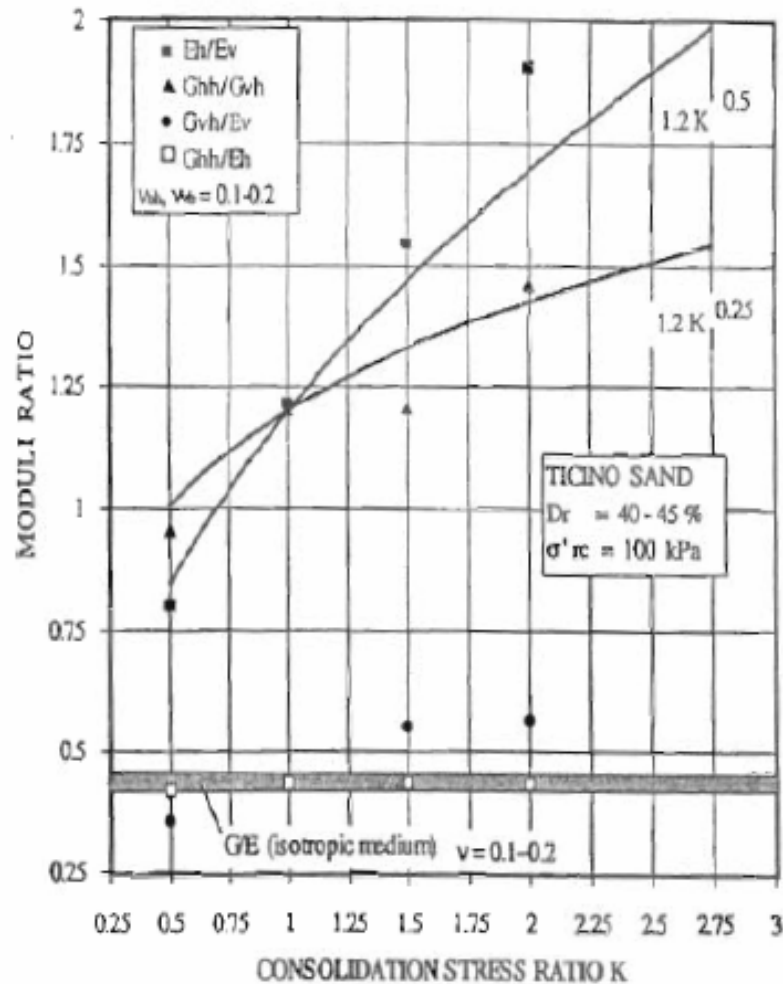


Fig. 9. Velocity of compression waves in an isotropically consolidated specimen

Fig. 10. Velocity of shear waves in an isotropically consolidated specimen

STRESS-INDUCED ANISOTROPY



- E_v = Young's modulus in the vertical direction
- E_h = Young's modulus in the horizontal direction
- ν_{vh} = Poisson's ratio for effect of the vertical stress on the horizontal strain
- ν_{hh} = Poisson's ratio for effect of the horizontal stress on complementary horizontal strain
- G_{vh} = shear modulus referred to the vertical plane.

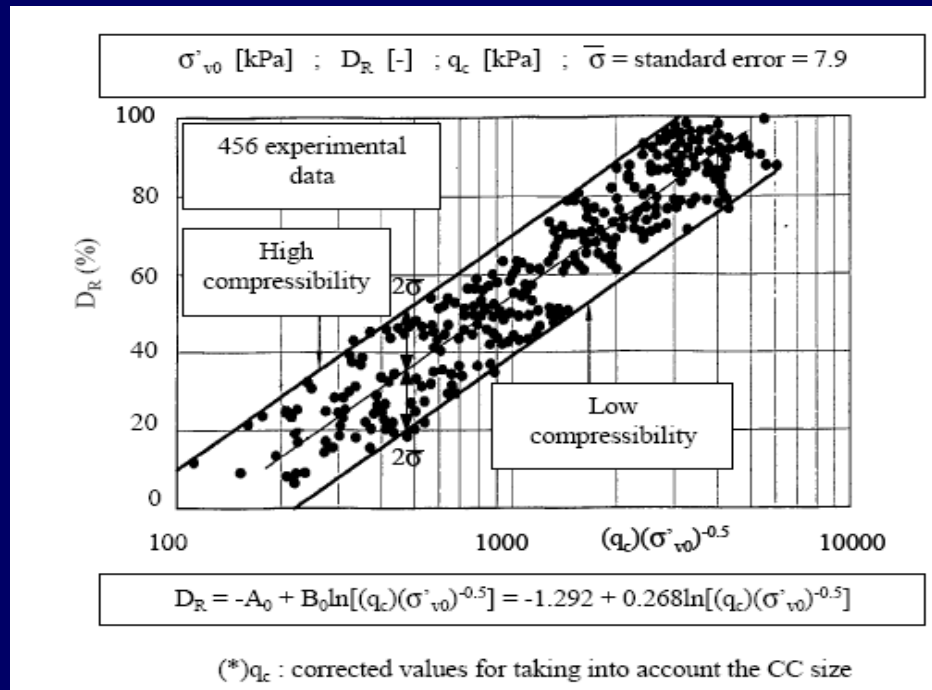
EFFECTS OF SATURATION ON SANDS

- CC DATA $S=0\%$ OR $S=100\%$ (FINE TO MEDIUM SANDS)
- NEGLIGIBLE EFFECTS (BELLOTTI ET AL. 1988 ISOPT I; SCHMERTMANN 1976, JAMIOLKOWSKI ET AL. 2001)



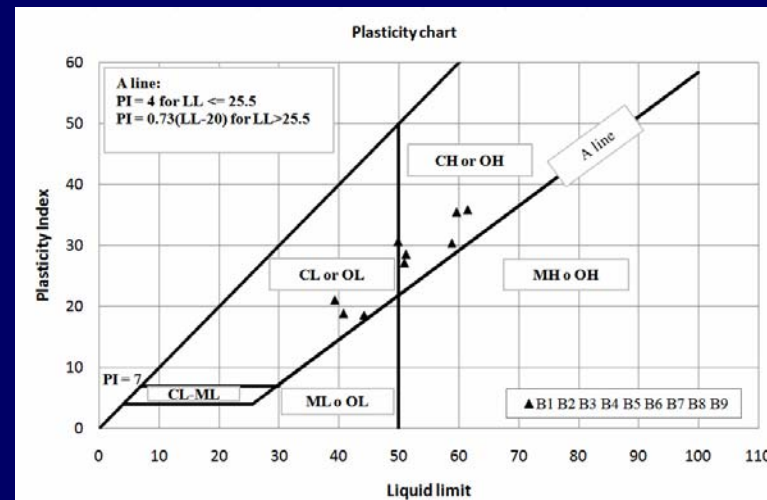
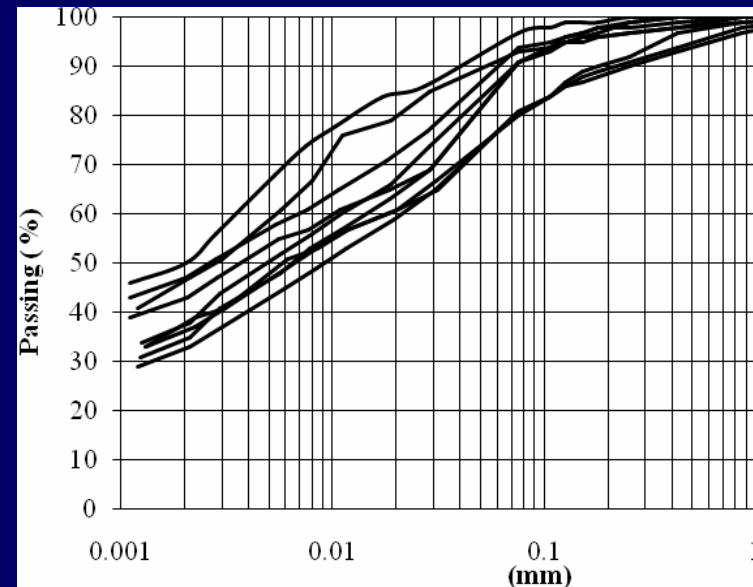
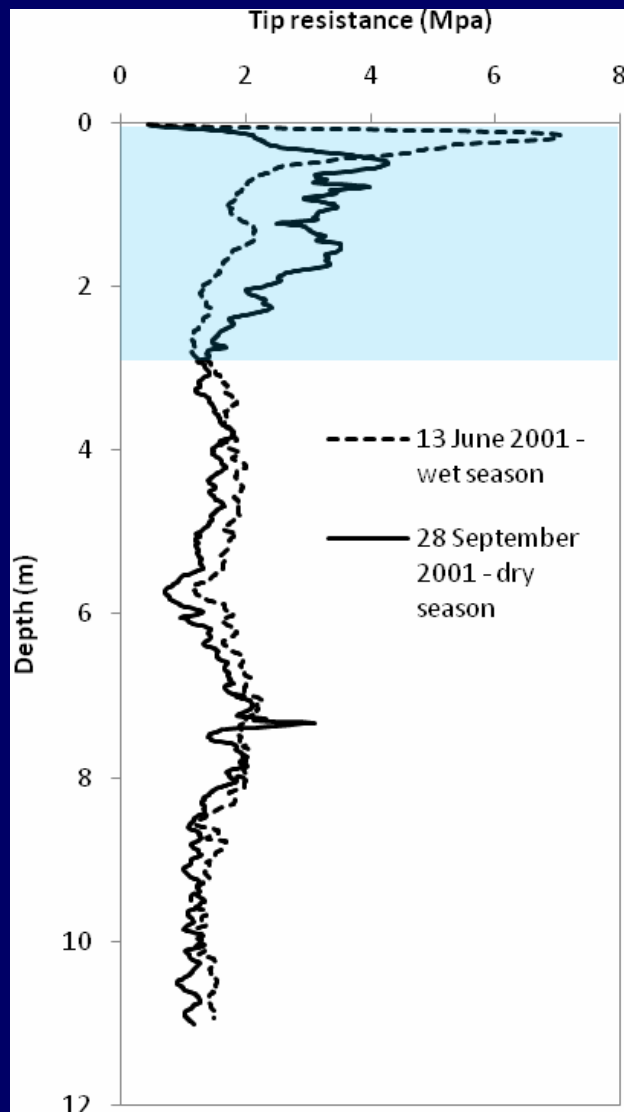
EFFECTS OF SATURATION ON SANDS

- JAMIOLKOWSKI ET AL. 2001
(UNDERESTIMATE 7 – 10%)

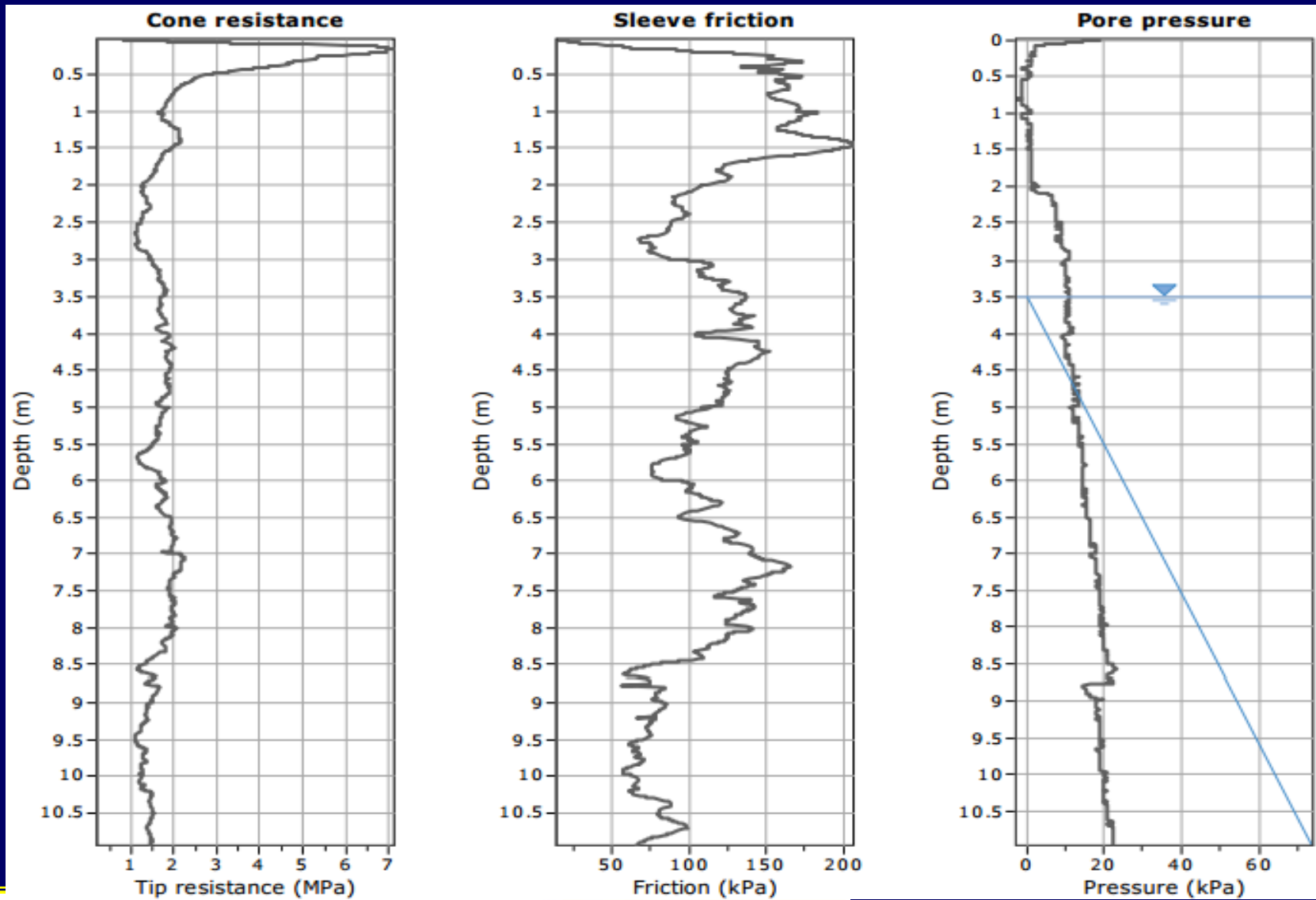


$$\frac{D_R(\text{saturated}) - D_R(\text{dry})}{D_R(\text{dry})} 100 = -1.87 + 2.32 \ln \frac{q_c}{(\sigma'_{v0} P_a)^{0.5}}$$

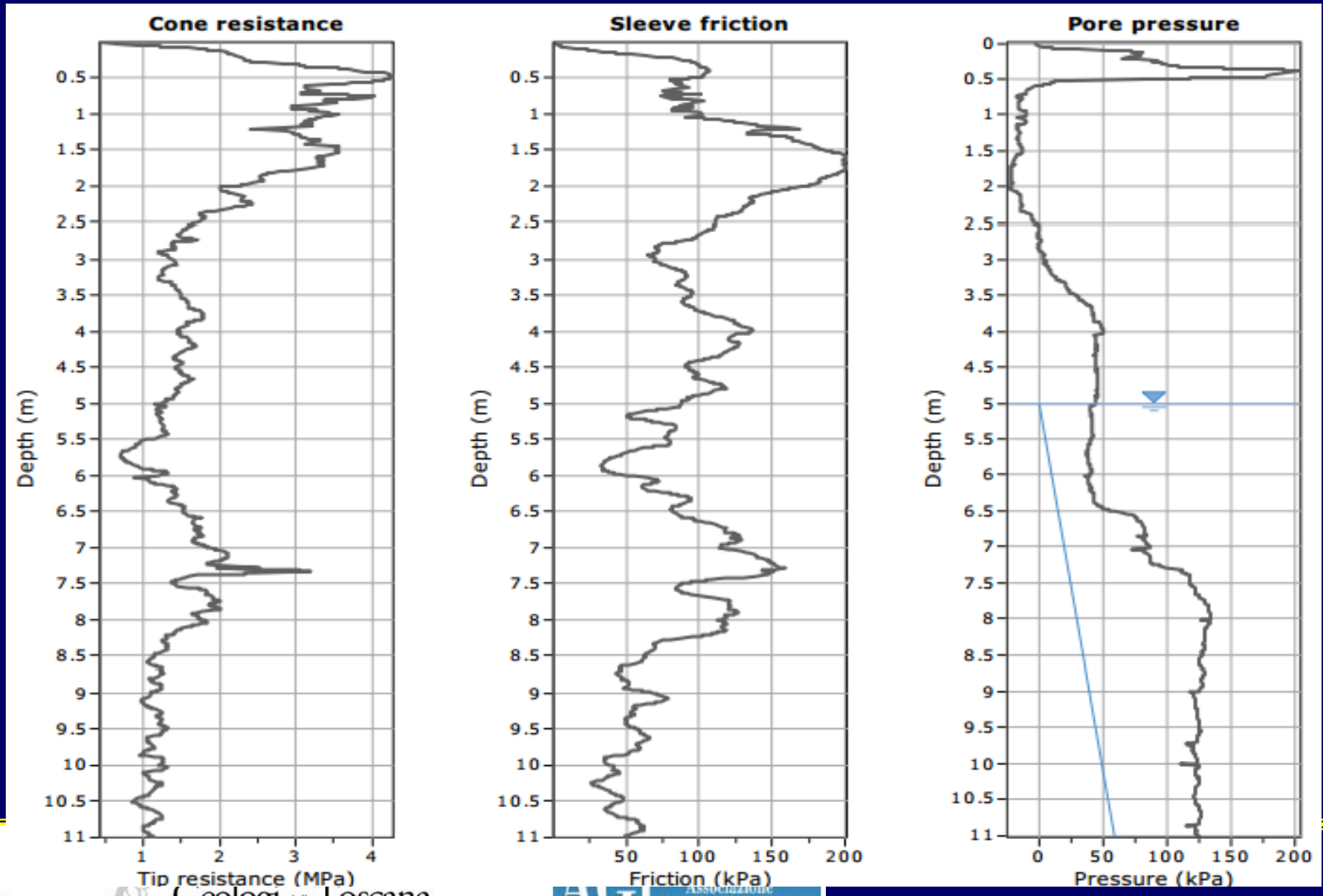
PARTIALLY SATURATED FINE GRAINED SOILS - BRONI



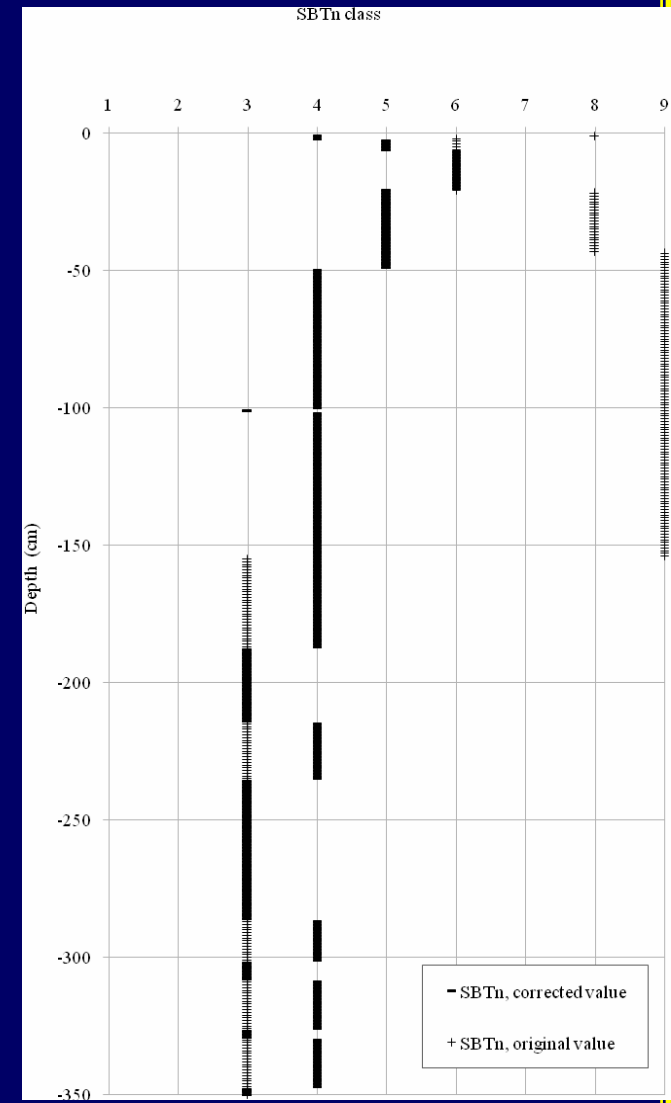
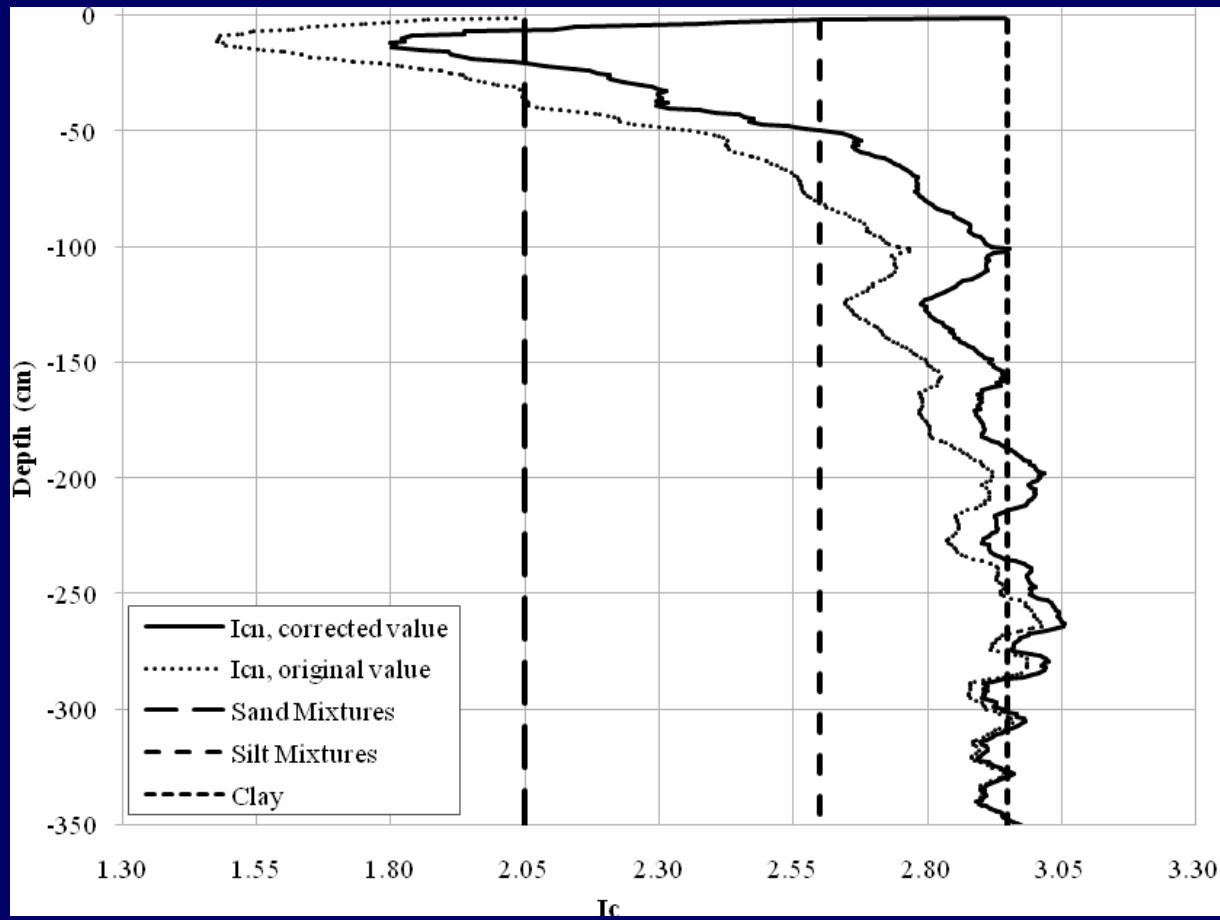
CPTu1 TEST CONDUCTED DURING THE WET SEASON (BRONI).



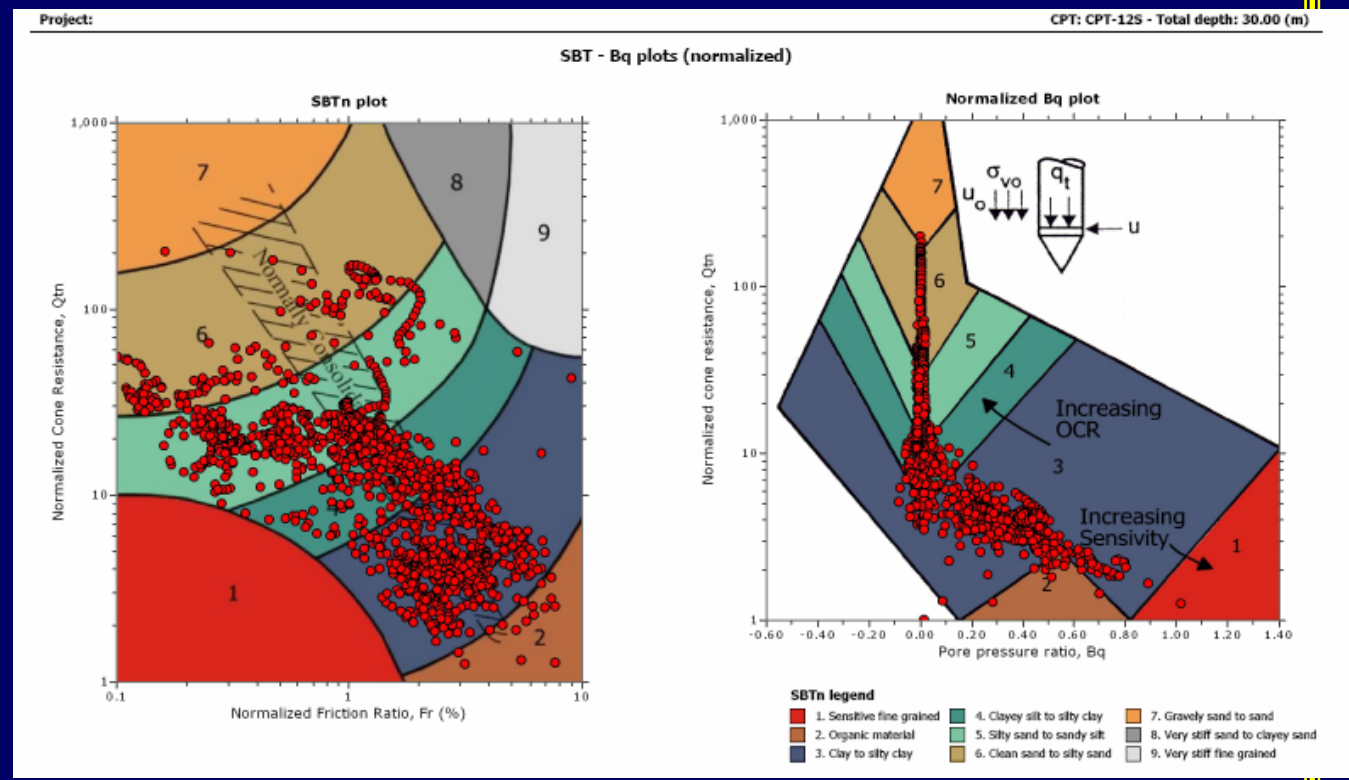
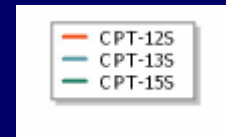
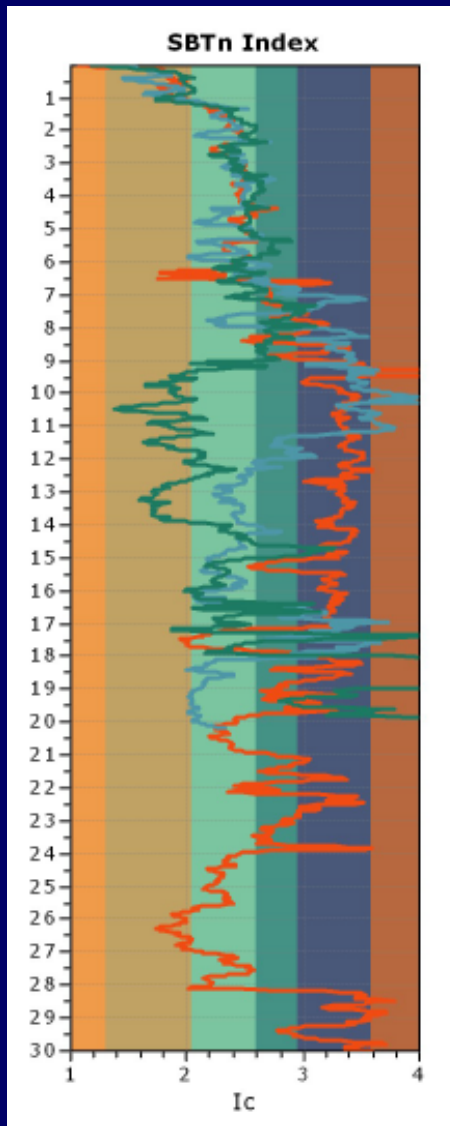
CPTu2 TEST CONDUCTED DURING THE DRY SEASON (BRONI).



MISS CLASSIFICATION AT BRONI



MISS – CLASSIFICATION: LOOSE SILTY SOILS (SERCHIO –RIVER LEVVES)



MK MODEL

- Kovacs (1981) and Aubertin, et al. (2002). The MK model has been used to evaluate, from simple soil parameters, the matrix suction (ψ_r) at the residual water content and the equivalent capillary height above the water table (h_{c0}).
- (Aubertin et al. 1998; Mbonimpa et al. 2000, 2002).

EQUATIONS

- For clayey soil

$$\psi_r = 0.86 \left(\frac{\xi}{e} \right)^{1.2} w_L^{1.74}$$

- ($\rho_s =$ dry density)

$$\xi \text{ (cm)} \approx 0.15 \rho_s \text{ (kg/m}^3\text{)}$$

$$u = -\gamma_w \cdot h \quad \text{for } 0 < h < h_{c0}$$

$$u = -\gamma_w \cdot h_{c0} \quad \text{for } h > h_{c0}$$

EFFECTS OF EFFECTIVE STRESS ON I_c

$$I_c = \sqrt{(3.47 - \log Q_{cn})^2 + (\log F + 1.22)^2}$$

$$Q_{cn} = \left(\frac{q_t - \sigma_{v0}}{\sigma_{atm}} \right) \left(\frac{\sigma_{atm}}{\sigma'_{v0}} \right)^n$$

$$F = \frac{f_s}{q_t - \sigma_{v0}} \cdot 100;$$

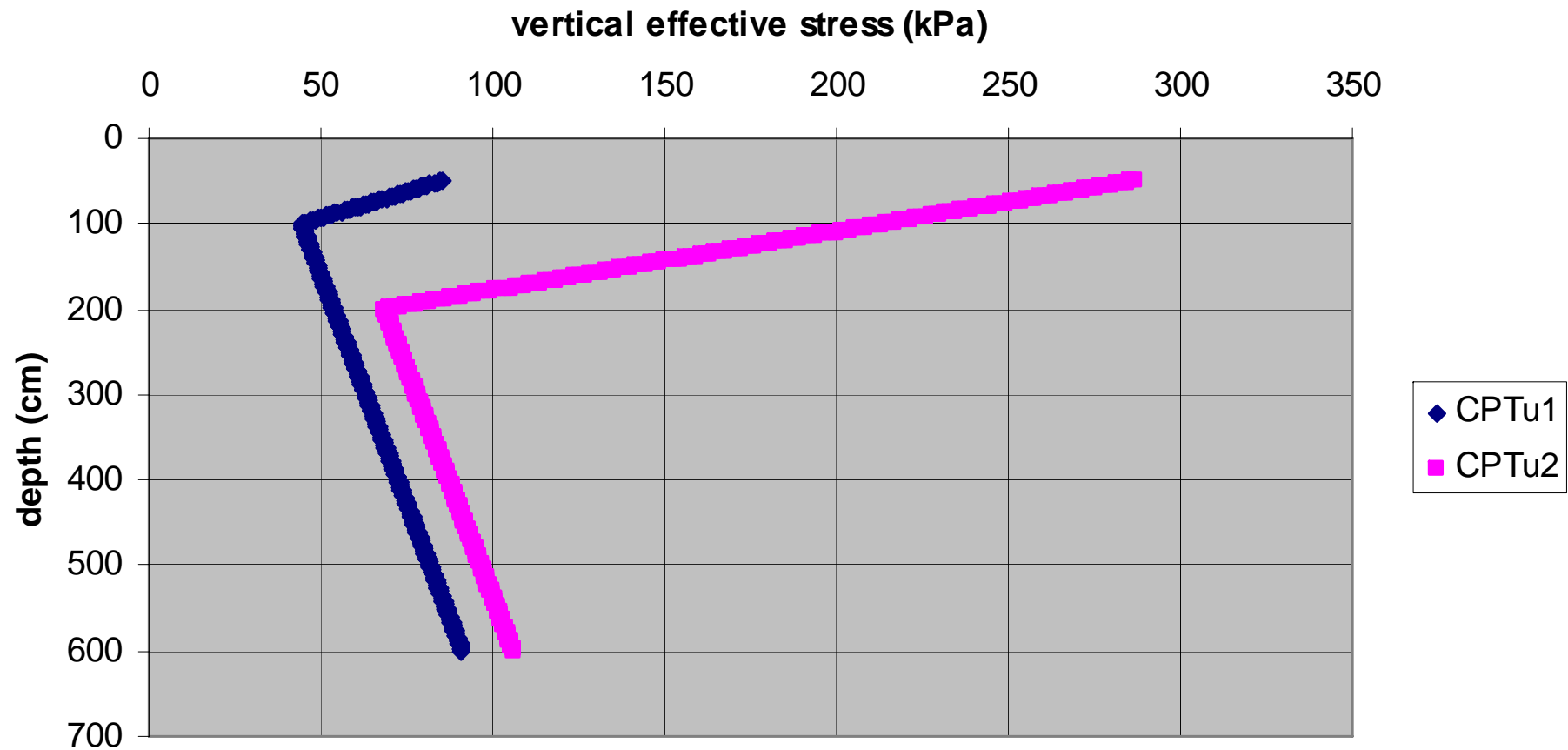
$$n = 0.381 \cdot I_c + 0.05 \cdot \left(\frac{\sigma'_{v0}}{\sigma_{atm}} \right) - 0.15$$

Soil classification (SBTn)	Zone number (Robertson SBT 1990)	SBT Index values
Organic soils: peats	2	$I_c > 3.60$
Clays: silty clay to clay	3	$2.95 < I_c < 3.60$
Silt Mixtures: clayey silt to silty clay	4	$2.60 < I_c < 2.95$
Sand Mixtures: silty sand to sandy silt	5	$2.05 < I_c < 2.60$
Sands: clean sand to silty sand	6	$1.31 < I_c < 2.05$
Gravelly sand to dense sand	7	$I_c < 1.31$



MODIFICATION OF THE VERTICAL EFFECTIVE STRESS TO GET A CONSTANT I_c (BRONI)

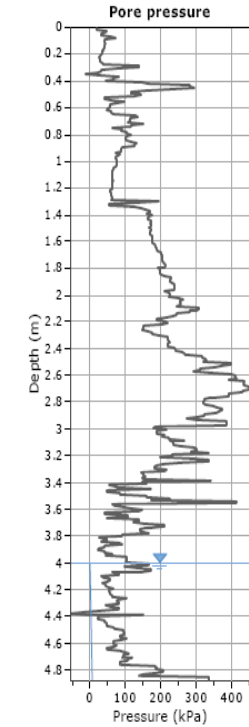
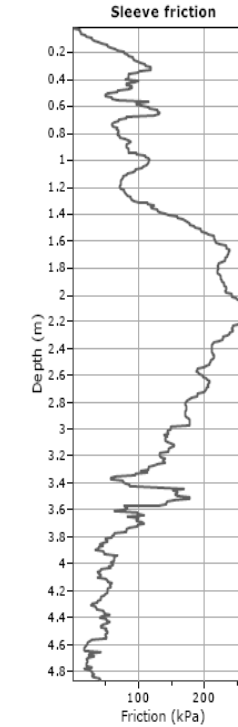
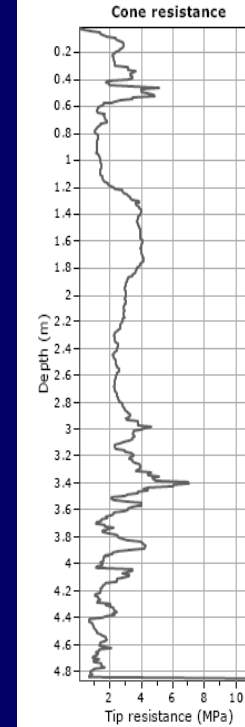
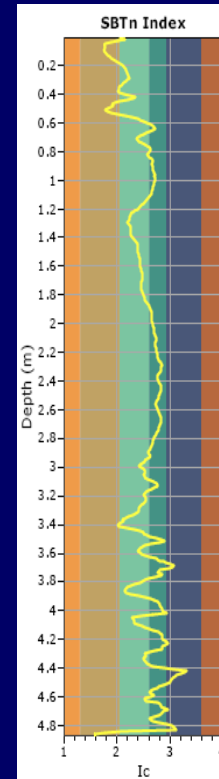
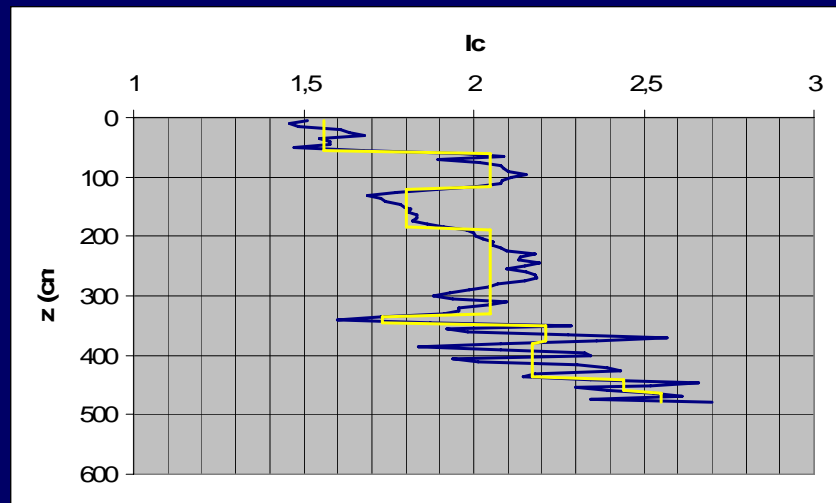
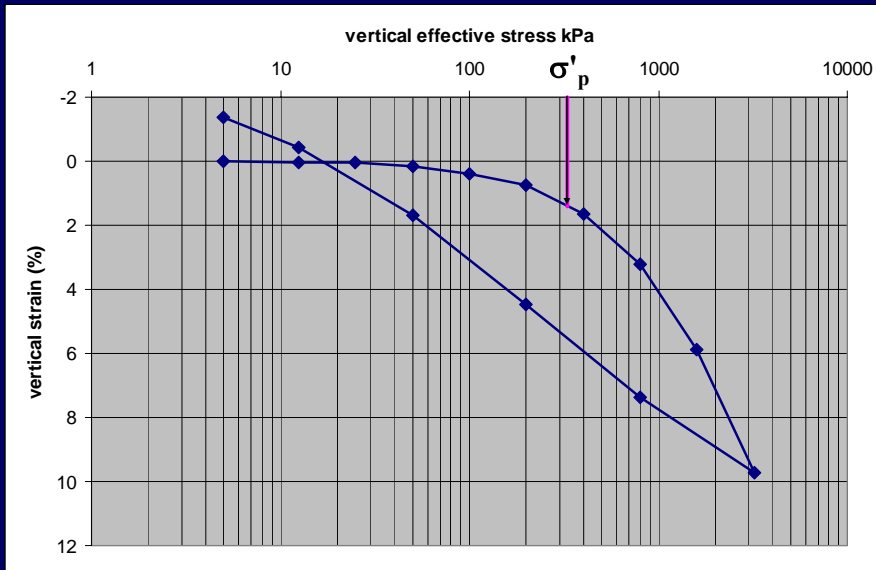
Estimate of vertical effective stress at Broni



PRACTICAL MEANING OF THE CORRECTION

- THE SBT INTERPRETATION COULD GIVE AN ASSESSMENT OF THE EFFECTIVE STRESS-STATE
- MODIFYING A SBT INTERPRETATION INTO A “SOIL TYPE” INTERPRETATION, THE DIFFERENCES IN I_c SHOULD REFLECT WHAT IS MISSING IN “SOIL TYPE” INTERPRETATION (i.e.OCR)

POSSIBLE IDENTIFICATION OF PRECONSOLIDATION PRESSURE (PORCARI LUCCA)



Preconsolidation pressure
From oedometer: 320 – 476 kPa
From CPTu: 296 kPa

EMPIRICAL CORRECTION

- Define I_c from borehole data (grain size curve)
- The new classification system is not SBT but depends on texture
- Criteria:
 - AGI 1997
 - Example: Silt with clay ($2.6 < I_c < 2.95$)

ASSESSMENT OF I_c FROM BOREHOLE

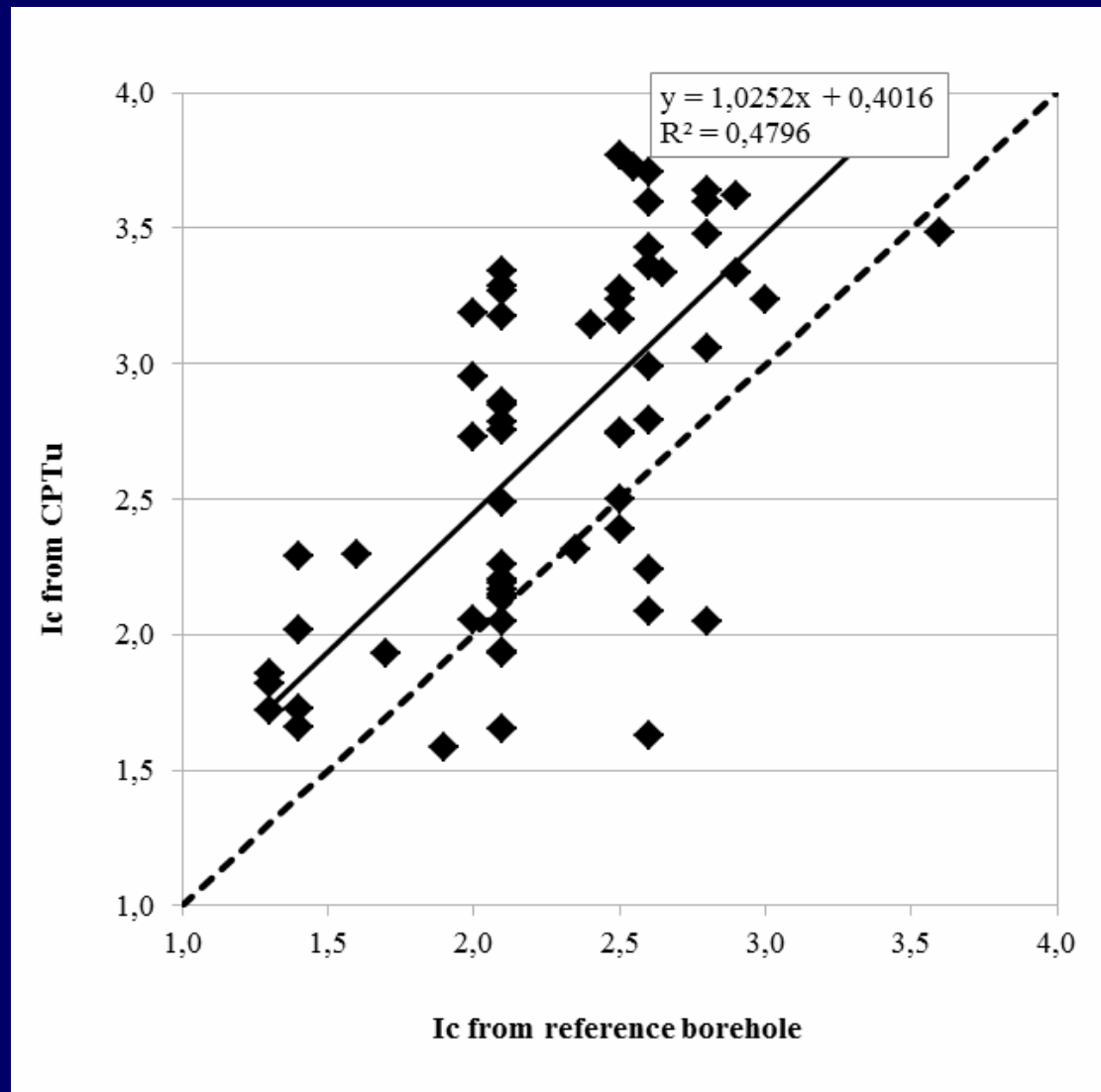
Borehole #	Soil classification from borehole (AGI 1997)	I_c from borehole	I_c from CPTu	ΔI_c	SBTn	Soil classification from CPTu
1	Clayey and sandy silt	2.60	2.79	0.19	4	Sand Mixtures
	Silty sand	2.10	2.05	-0.05	6	Sand
	Silty sand	2.10	2.49	0.39	5	Sand Mixtures
	Sand, gravel and fine gravel	1.30	1.72	0.42	6	Sand
	Silty sand	2.10	2.19	0.09	5	Sand Mixtures

DATABASE

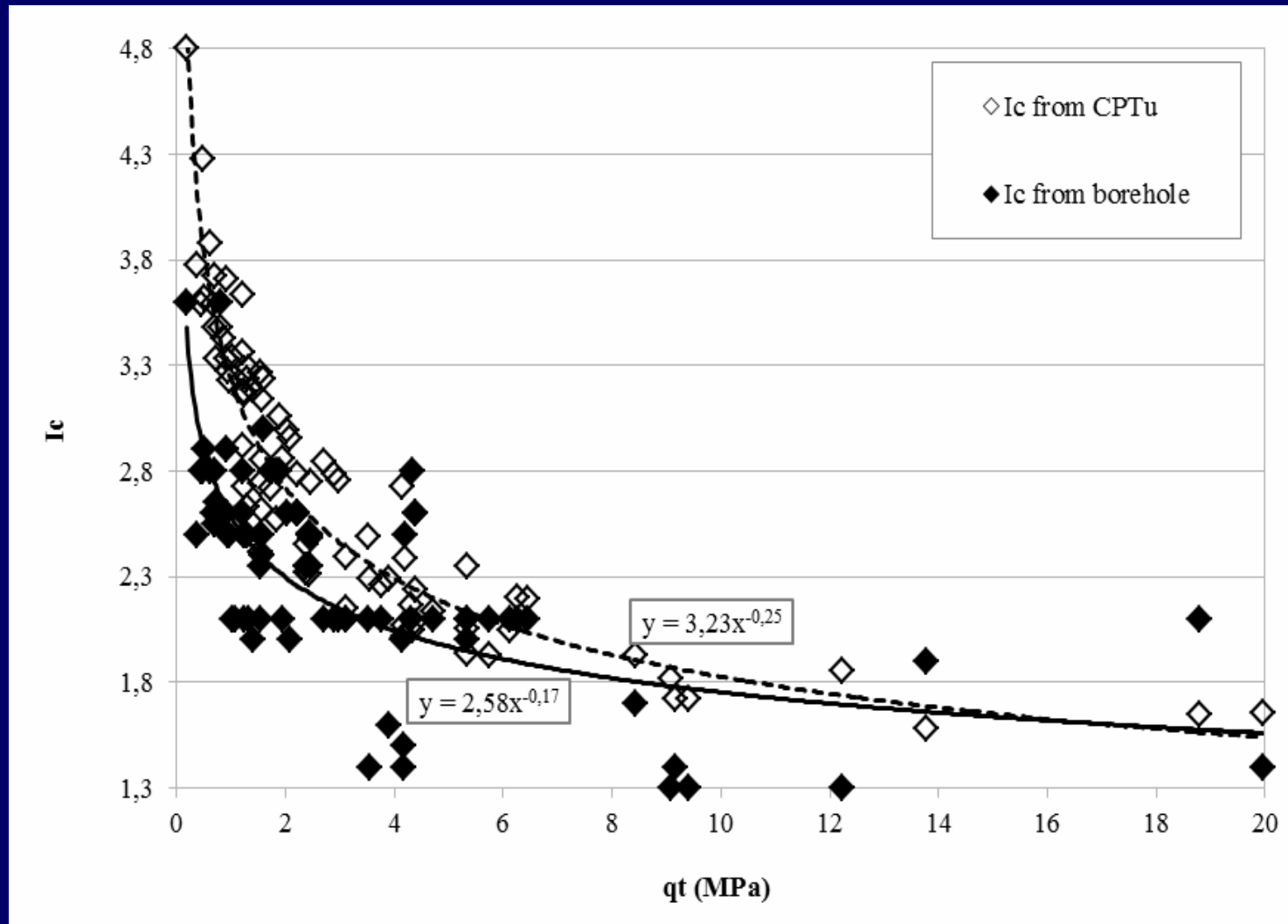
- SERCHIO RIVER LEVEE SYSTEM AND FOUNDATION SOIL
- DREDGED SEDIMENTS IN ARTIFICIAL BASINS (PORT OF LIVORNO)



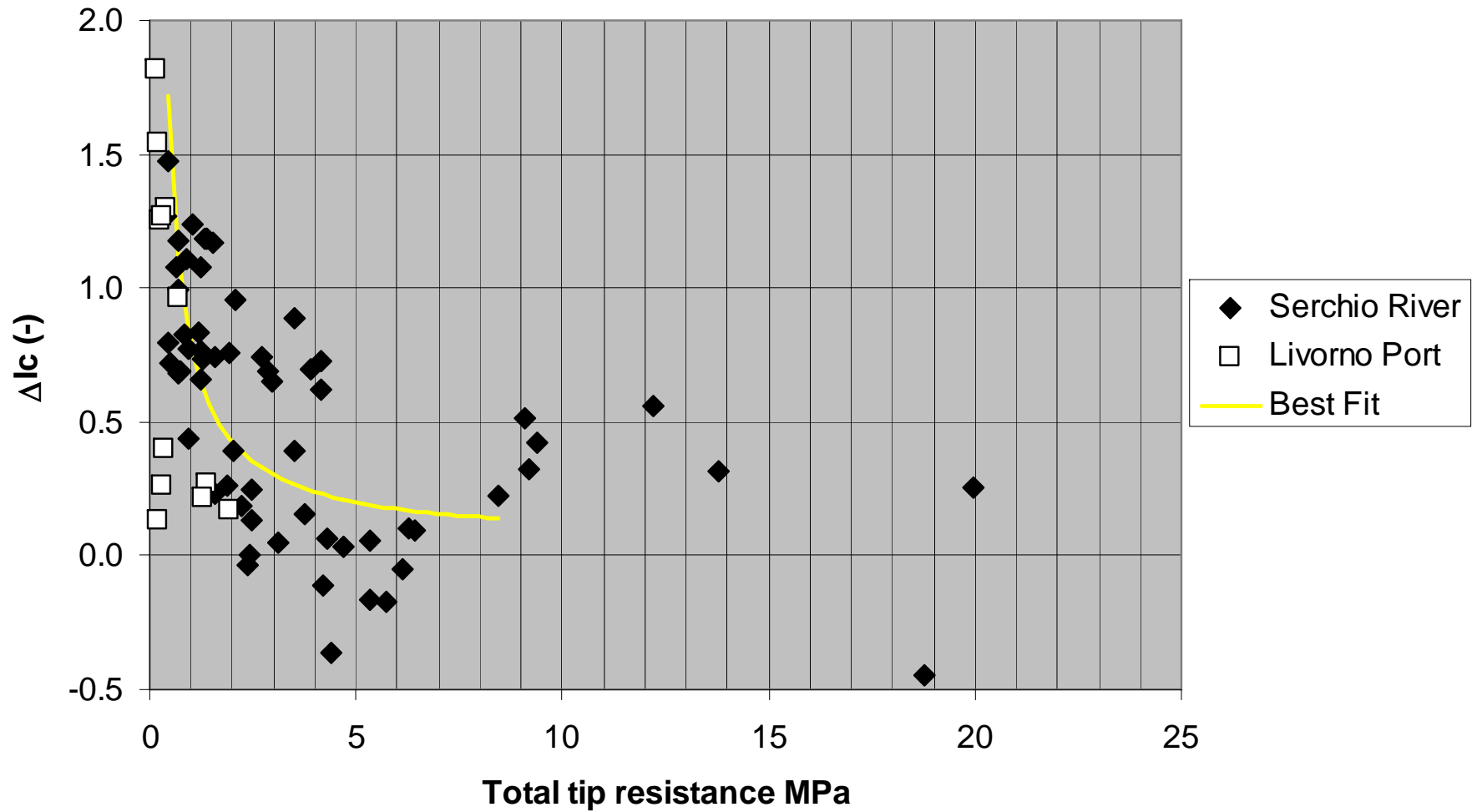
Ic FROM BOREHOLES AND CPTu



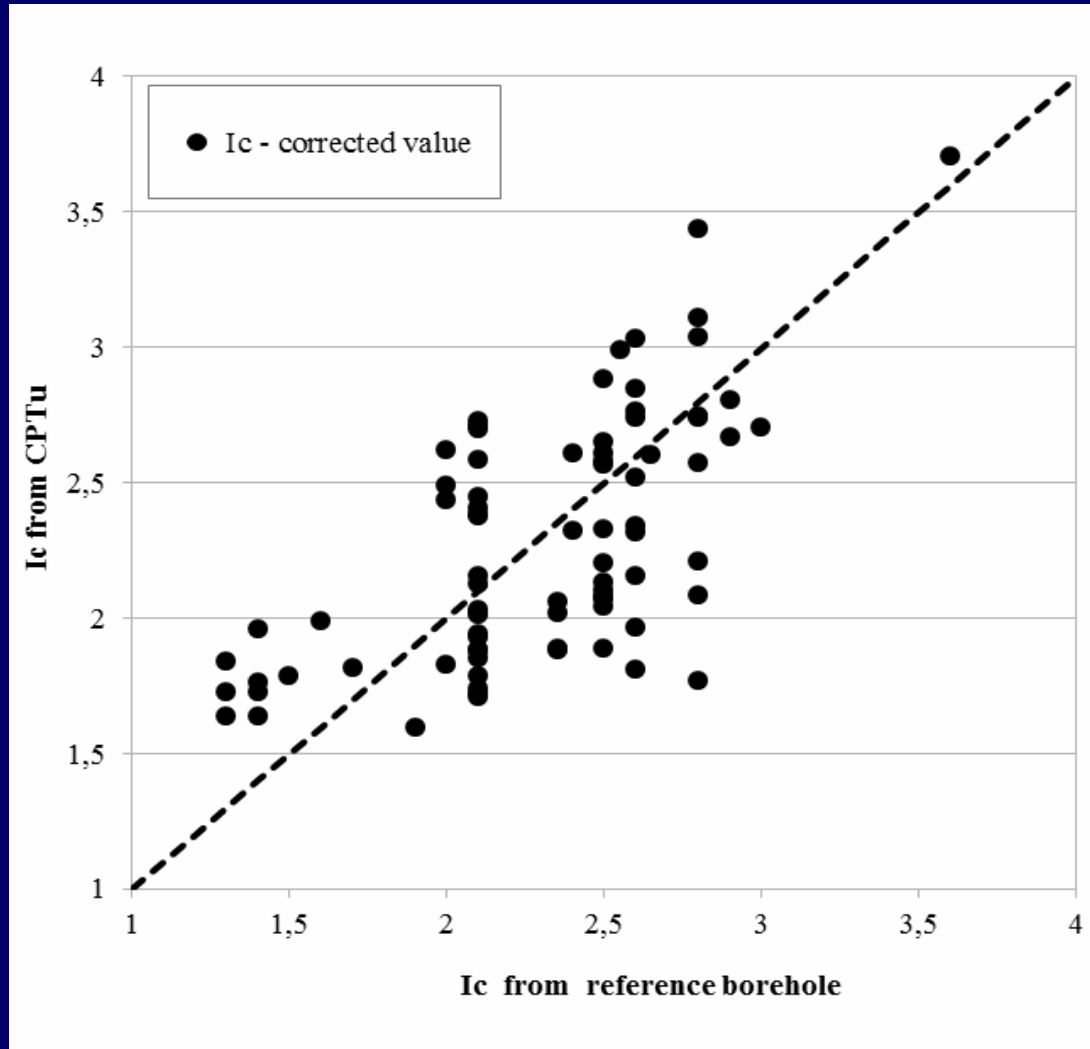
HOW TO CORRECT



ΔI_c FUNCTION



Ic AFTER CORRECTION



ACKNOWLEDGEMENTS

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