

The CPT and some additional Sensors

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JMP

Pisa 14th June 2016

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ADDITIONAL SENSORS THAT CAN BE INCORPORATED



- Modern electronics, sensor technology and data acquisition systems have opened up a whole new world for 'add-on' devices to the CPT/CPTU.
- We can now supplement the information from a CPT or CPTU by adding additional sensors.

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Add on devices

- Lateral stress measurements
- Cone Pressuremeter
- Seismic measurement
- Electrical resistivity
- Heat flow
- Density probes
- Acoustic noise
- Vision/video cone
- Gamma cone
- Magnetometer

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Seismic Measurements

- Martin Fahey (2001) said that the addition of seismic measurements to the CPTU should become the next standard/routine form of the CPTU.

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Seismic Measurements

- Martin Fahey (2001) said that the addition of seismic measurements to the CPTU should become the next standard/routine form of the CPTU.
- We have come a long way and I think this is now proving to be the case

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Seismic cone

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Or Small strain shear modulus and the CPT

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Small strain shear modulus

- The shear modulus is largest at very low/small strains and has received particular attention in recent time.
- This initial, small strain, modulus is often denoted G_0 or G_{\max} (this may lead to some confusion as will be discussed later)

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Seismic measurements

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Seismic Piezocone = SCPTU

Add geophones and/or accelerometers to CPTU to measure arrival of compression wave (P) and shear wave (S) to compute the compression wave velocity (V_p) and the shear wave velocity (V_s)

Elastic theory (since strains induced in the soil by the waves are very small) allows for computation of the modulus parameters:

- Small Strain Shear Modulus = $G_0 = G_{max} = \rho_t(V_s)^2$

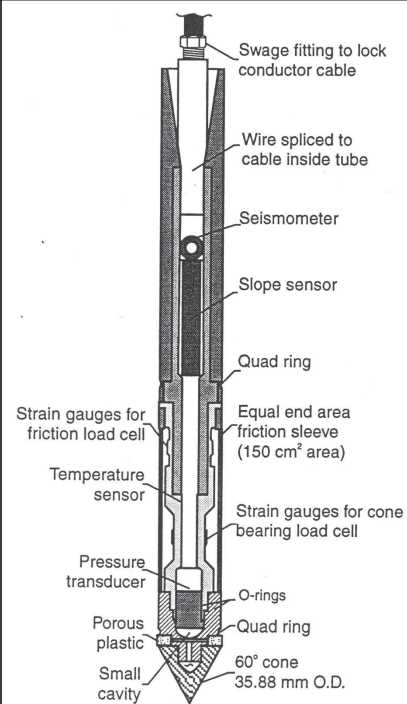
- Constrained Modulus = $M_0 = \rho_t(V_p)^2$

ρ_t = total unit weight



Seismic Cone

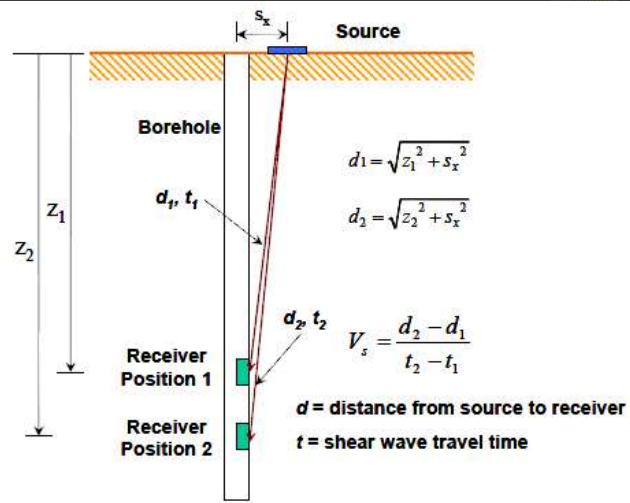
Can be 3D arrays



SCPT test

Energy source at the ground surface initiates the waves, sensors in the cone body (usually just short distance after the friction sleeve) detect the wave arrival.

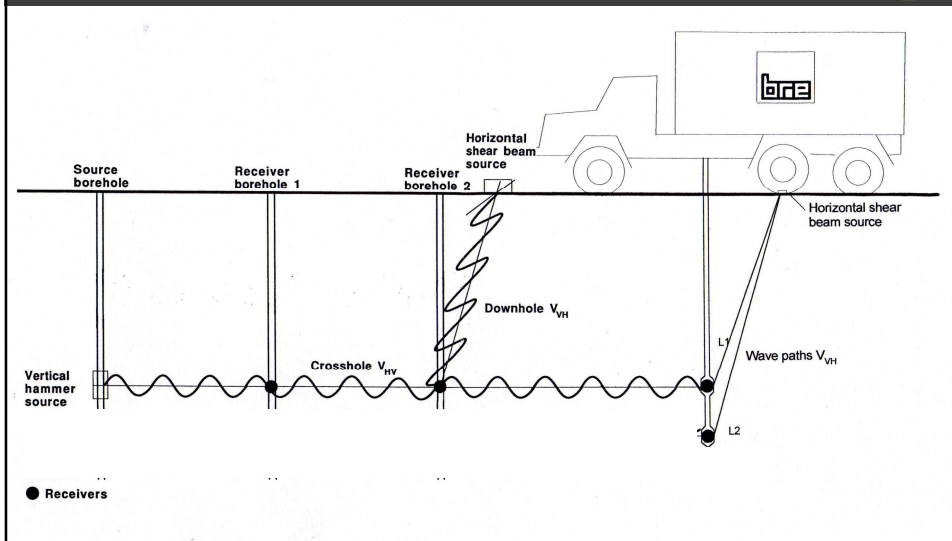
Source energy can be activated manually (e.g., hammer) or semi-automatically (e.g., hydraulic system).



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Geophysical testing



Mechanical Seismic Source – Cone Truck



Normal force applied to beam for good contact with ground surface

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Mechanical Seismic Source – Cone Truck - Automated



Normal force applied to beam for good contact with ground surface

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SCPTU – "Portable" Source Beam for use with Drill Rigs



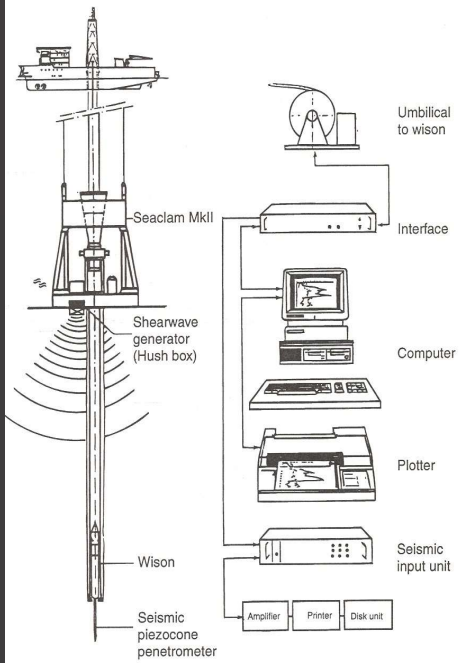
Trigger for recording of $t = 0$

Set-up allows for reverse strikes

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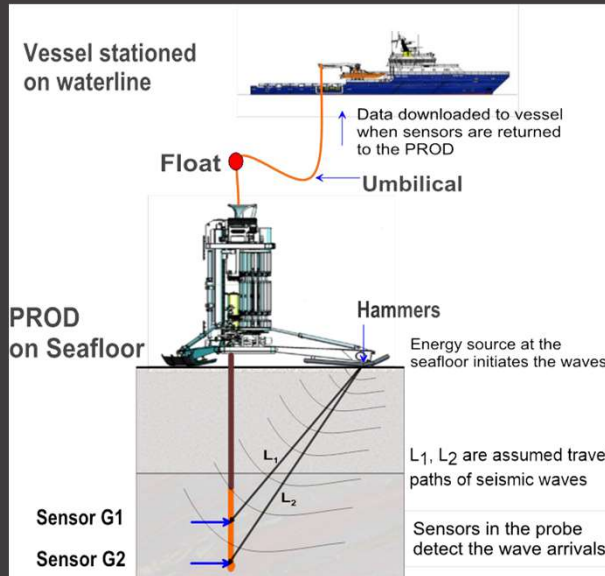
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Seismic Cone



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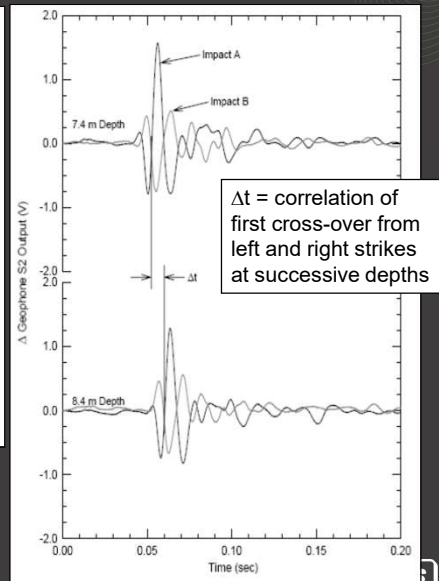
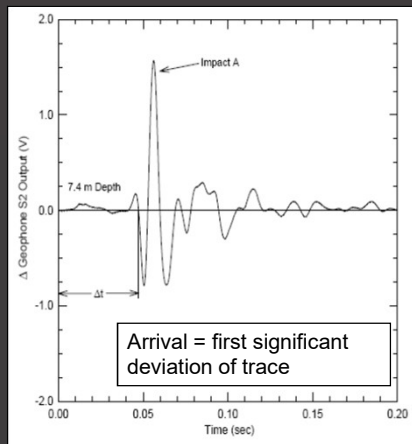
PROD SEISMIC PROBE



MPMP Courtesy of Hoang, Benthic

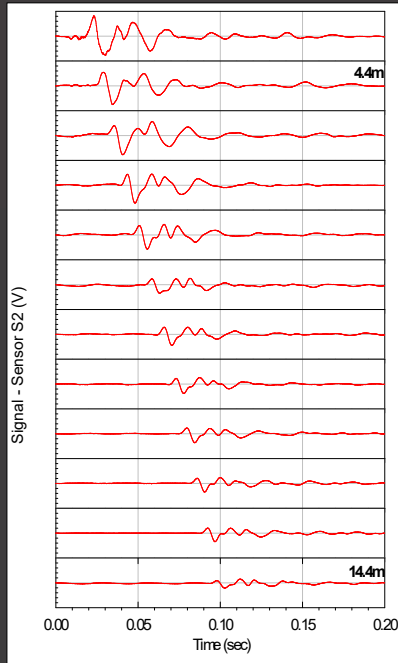


Example SCPTU Traces – Boston Blue Clay



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Example SCPTU Data



Boston Blue Clay –
Newbury, MA

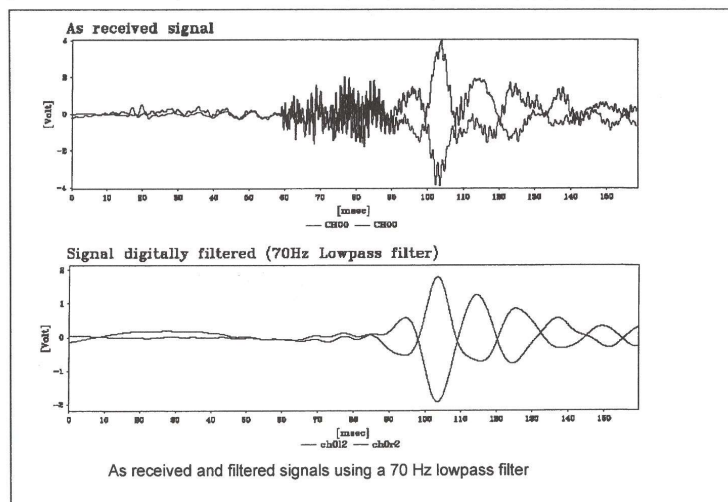
Seismic tests done at
each 1 m rod change

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Reversal

Seismic shear waves - Typical data

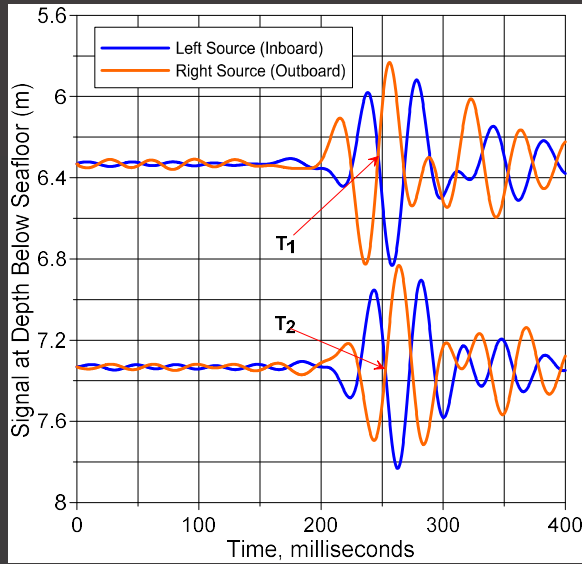


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DATA REDUCTION: Filtered Data - Offshore

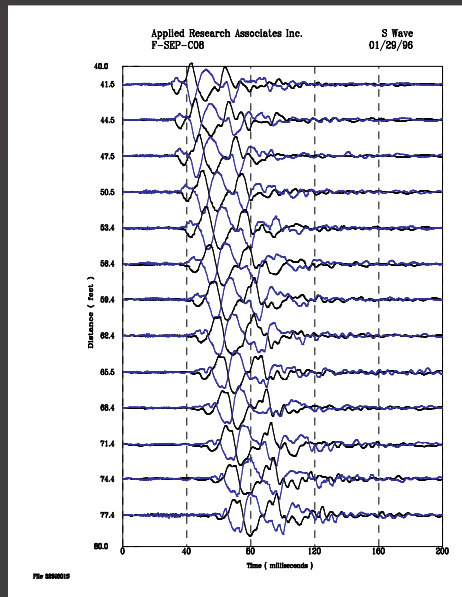
Slide 28



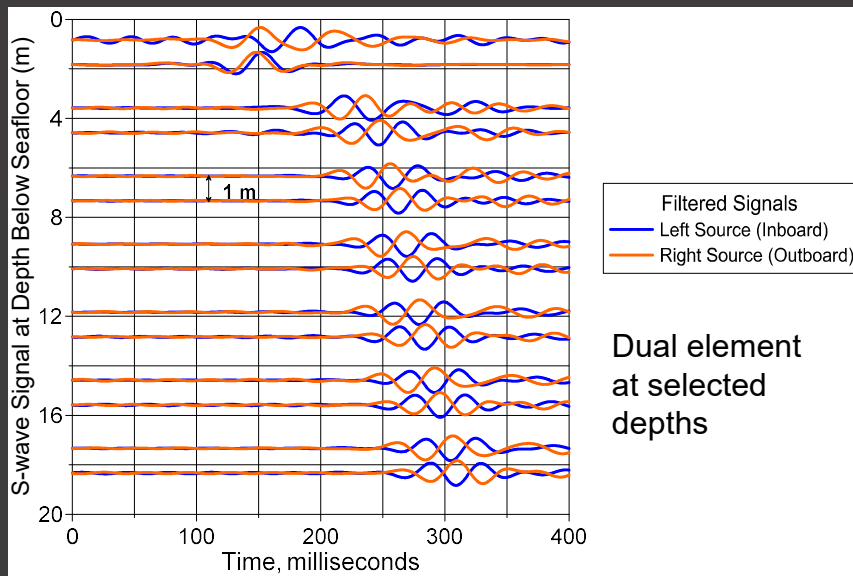
Courtesy of Hoang, Benthic



Example of Seismic-CPT Data



DATA REDUCTION: Filtered Data



JMP Courtesy of Hoang, Benthic

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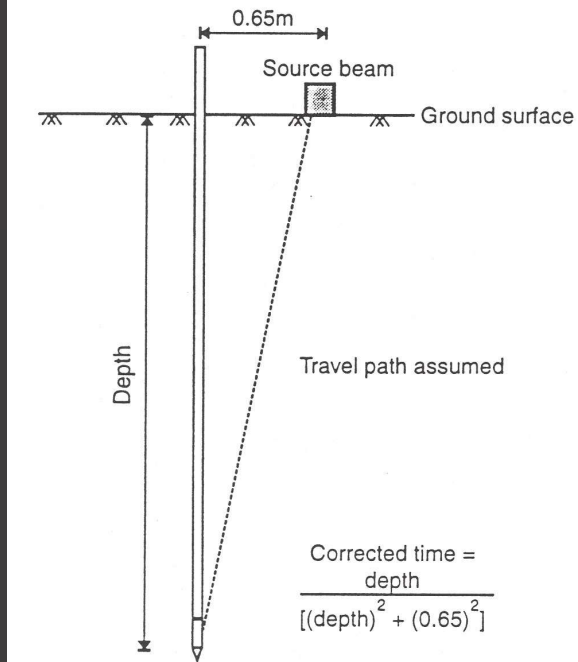
Potential Errors/problems

- Time
- Distance

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SCPT test

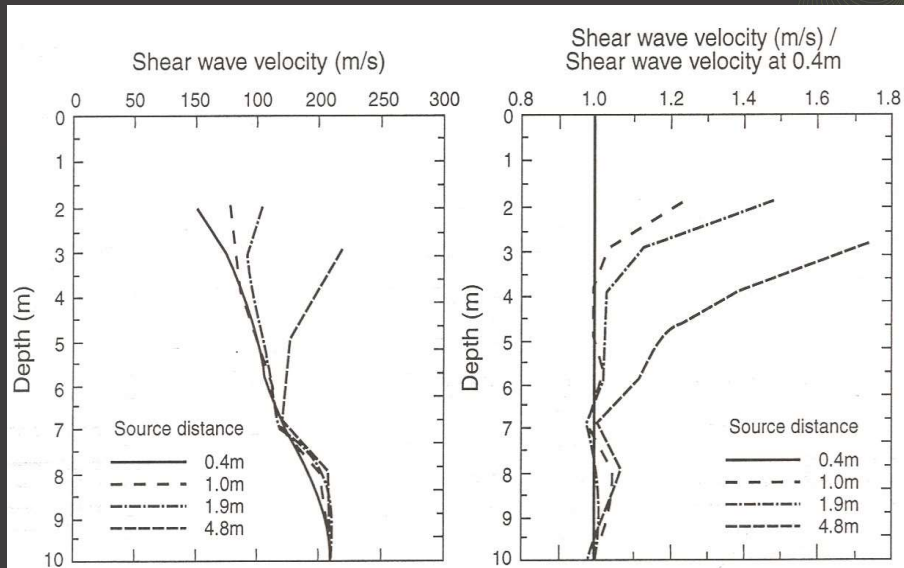


Dual element option etc

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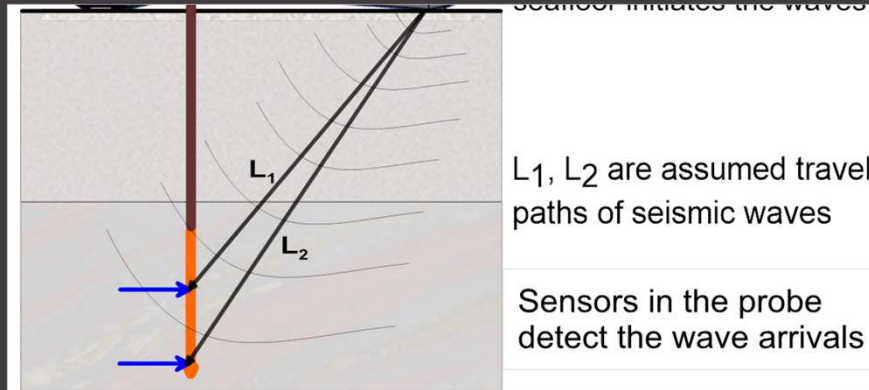
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Potential Errors



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PROD SEISMIC PROBE



L_1 , L_2 are assumed travel paths of seismic waves

Sensors in the probe detect the wave arrivals

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Data Reduction

Shear Wave Velocity: $V_s = \Delta L / \Delta t$

Measurement Methods:

1. Pseudo Interval – difference in arrival time between successive depths using single set of geophones
2. True Interval – two sets of geophones in the cone, measure arrive of same wave to directly determine Δt

Determination of Arrival time:

1. First deviation of the trace
2. Cross-correlation between successive depths
3. First cross over of wave traces when using left and right strikes

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Seismic Cone Testing

- | | | |
|---|----|---|
| <ul style="list-style-type: none"> • Dual
Spacing fixed
typically 0.5 -1m

Triggering same

Generally more reliable
Level of uncertainty removed | vs | <ul style="list-style-type: none"> Single element
dependent on push interval
accuracy of depth measurement

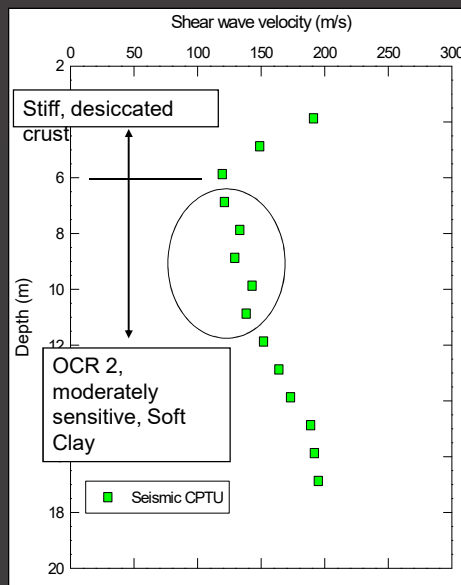
can have differences

peaks and troughs in data |
|---|----|---|

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Example vs Profile from SCPTU



Boston Blue Clay,
Newbury, MA

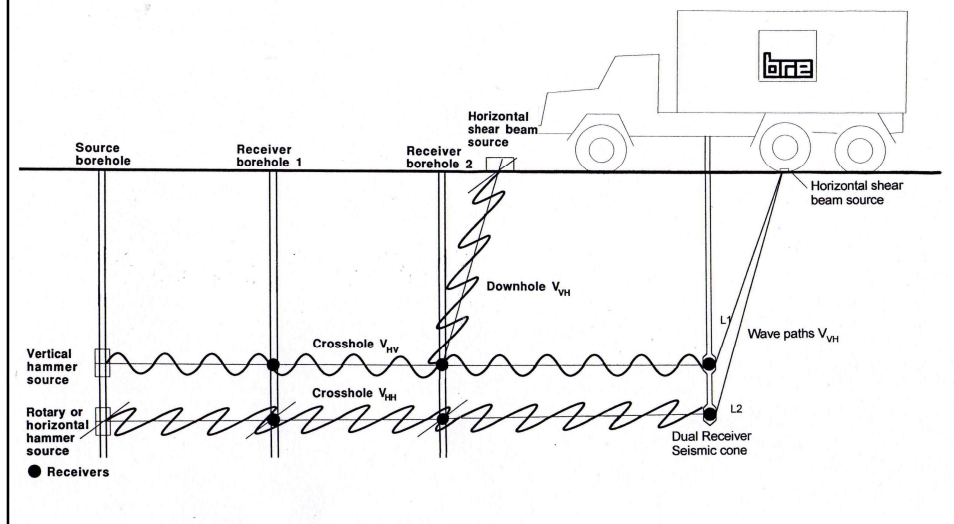
Used pseudo interval
method; analyzed
data via crossover
and cross-correlation
methods

With estimate ρ_t can
then convert to G_{max}
profile

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Geophysical testing - Anisotropy



Shear Wave Velocity - Fundamentals

The in situ shear wave velocity, V_s (and hence small strain shear modulus G_{max}) can be highly anisotropic. Thus direction of travel and polarization of wave is important.

V_{vh} – vertically propagating, horizontally polarized wave

V_{hh} – horizontally propagating, horizontally polarized wave

V_{hv} – horizontally propagating, vertically polarized wave.

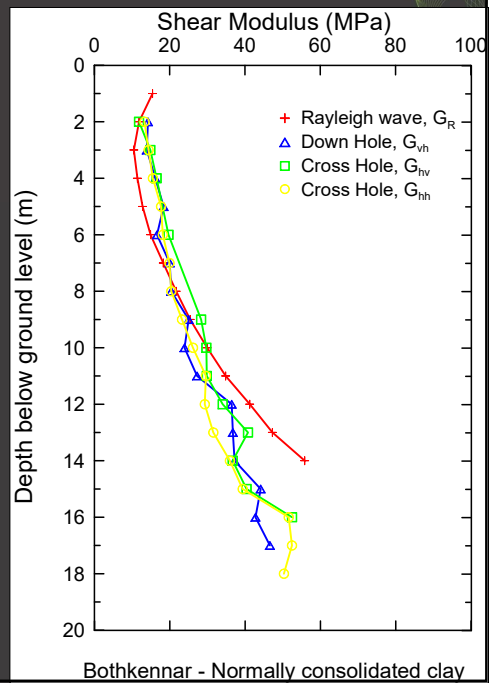
In some soils $V_{vh} \approx V_{hv}$; in most soils $V_{vh} \neq V_{hh}$.

SCPTU is a downhole method and thus measures V_{vh} or gives G_{vh}
(although most refer to SCPTU shear wave velocity as V_s)

Geophysics - Bothkennar

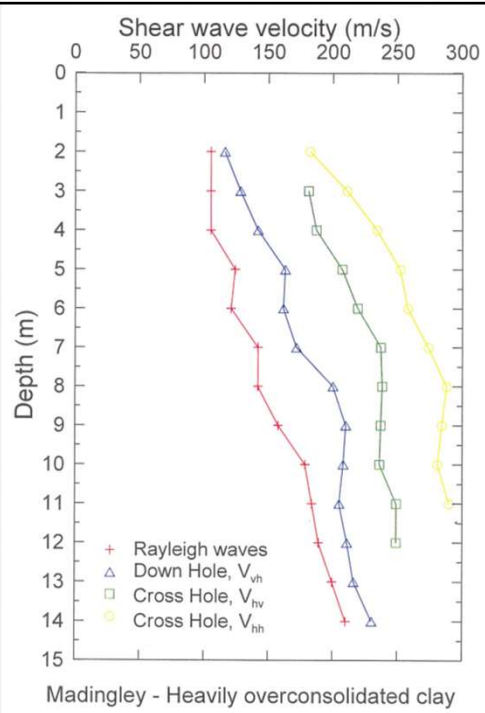
(so G_0 unique???)

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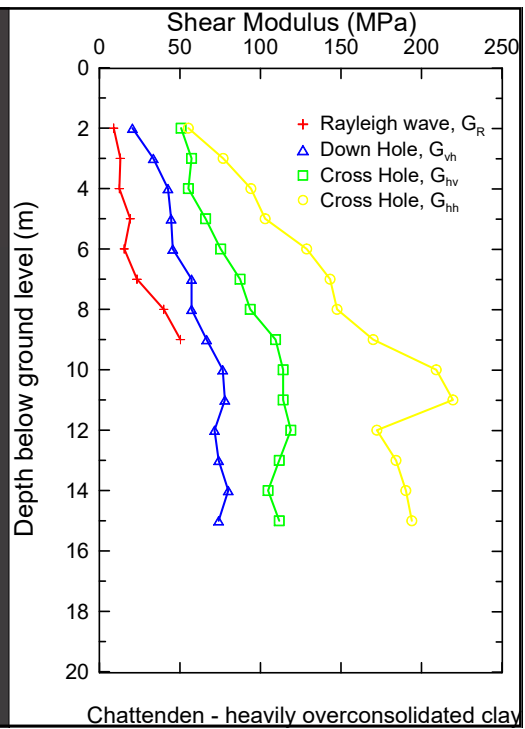
Shear wave velocities - stiff clay

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Geophysics - Heavily OC clay

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Seismic Cone

- But we know what we are measuring!! VH

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BUT !!!!!

- We always seem to like to find otherways of getting the same information from other data
- OR
- We like to better understand interactions in our parameters

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Basic Equation

- Correlations between index parameters and V_s or G_{max} can provide rapid estimates useful for preliminary design and for verifying *in situ* and laboratory results.
- (Hardin, 1978) suggested that G_{max} for clays depends on the *in situ* (or applied) stress (σ'), void ratio (e), and OCR.
- The empirical equation describing the influence of the controlling factors on G_{max} can then be written as follows:
- $$G_{max} = SF(e)(\sigma'_v \sigma'_h)^n p_a^{(1-2n)}$$
- where S is a dimensionless parameter characterizing the considered soil;
- $F(e)$ is a void ratio function;
- σ'_v and σ'_h are the vertical and horizontal effective stresses, respectively;
- n is a parameter indicating the influence of stress; and p
- is the atmospheric pressure

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BUT

- People seem to have tried to link G_0 with many other CPT parameters:
- q_c
- q_t
- q_{net}
- B_q
- f_s
- etc

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Small strain shear modulus

$$G_0 = 99.5 (p_a)^{0.305} \frac{(q_c)^{0.695}}{(e)^{1.130}}$$

where:

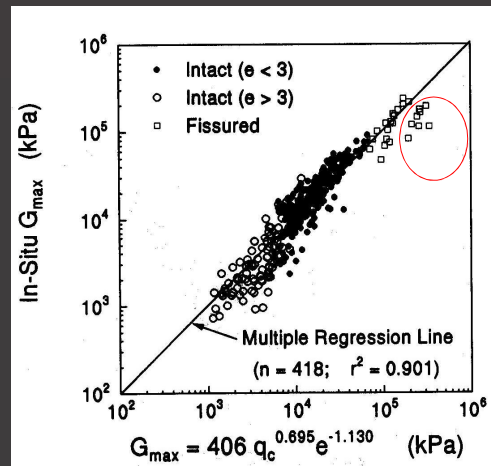
p_a = atmospheric reference stress in the same units as G_0 and q_t .

NOTE much of the original scatter could also have related to the use of q_c and not q_t

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Small strain shear modulus from CPTU data



From Mayne and Rix (1993)

What $G_{??}$

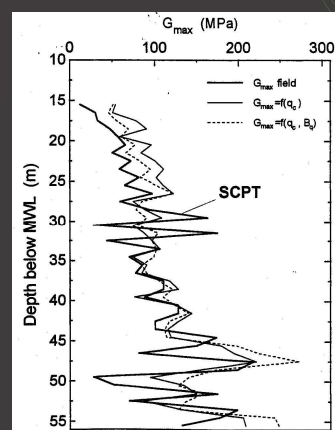
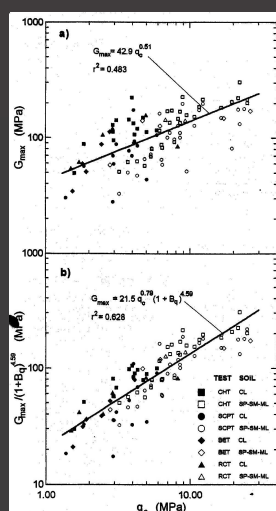
Do we believe?

See later

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$$(r_2 = 0.901) \quad G_{max} = 406(q_c)^{0.695}/e^{1.130}$$

CPTU - G_{max} correlations for Venetian soils -

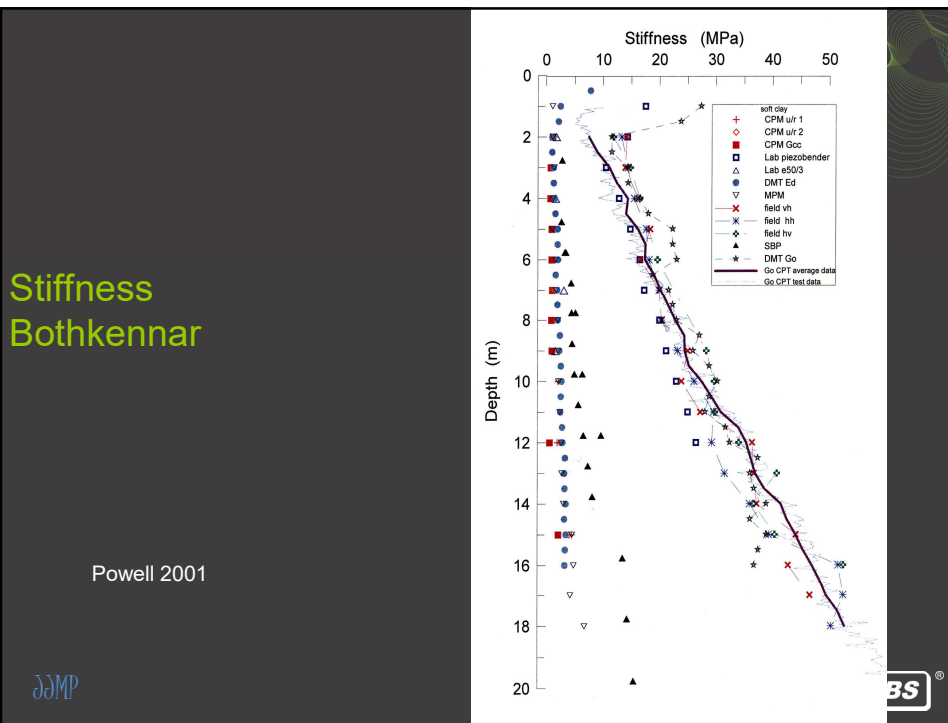


Comparison with G_{max} measured with seismic cone

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Correlations from Simonini and Cola, 2000



Equations

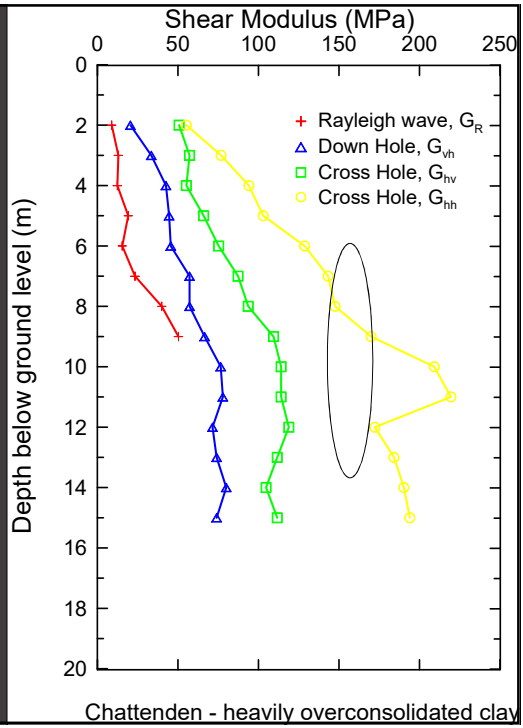
- All looks great
- But you find the same equations being modified for different sets of soils
- Eg. Recent Long and Donohue used the Simonini and Cola (2000) but had to modify it to get better fit for Norwegian soils (nc to lightly oc!!!)
- Care needs to be taken if correlations are used in different situations

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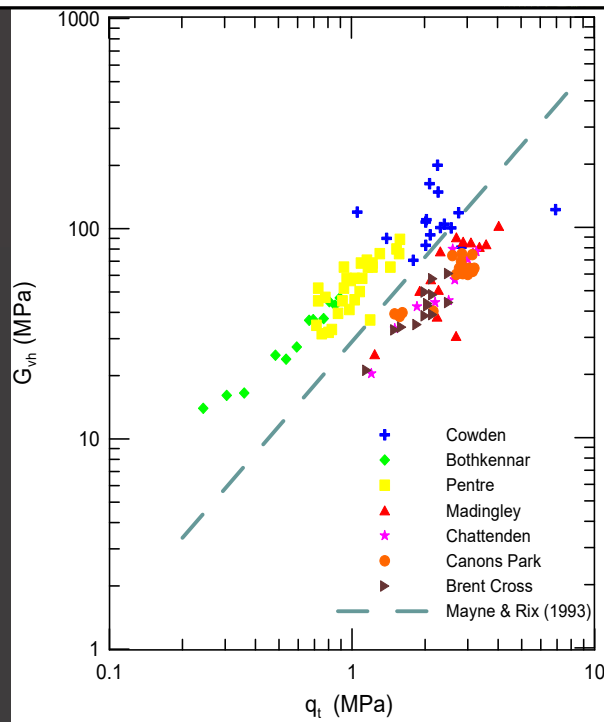
Geophysics - Heavily OC clay

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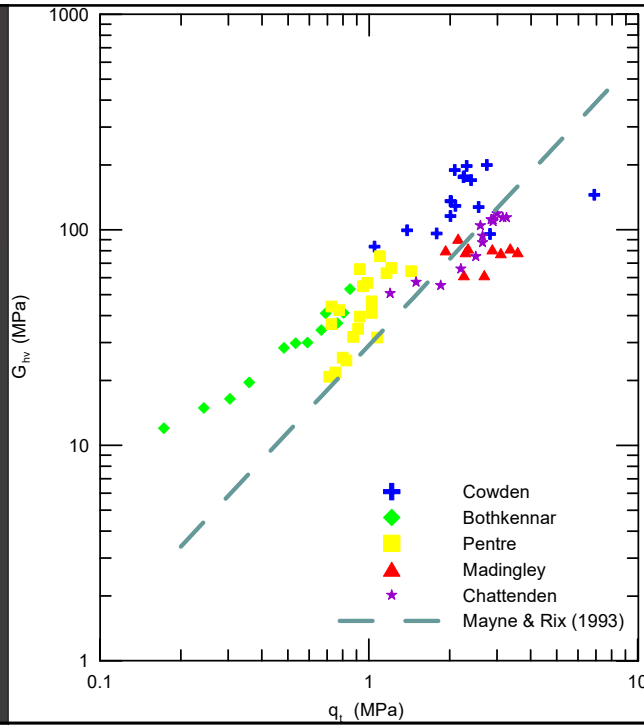
G_{vh} in UK soils

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G_{HV} in UK soils

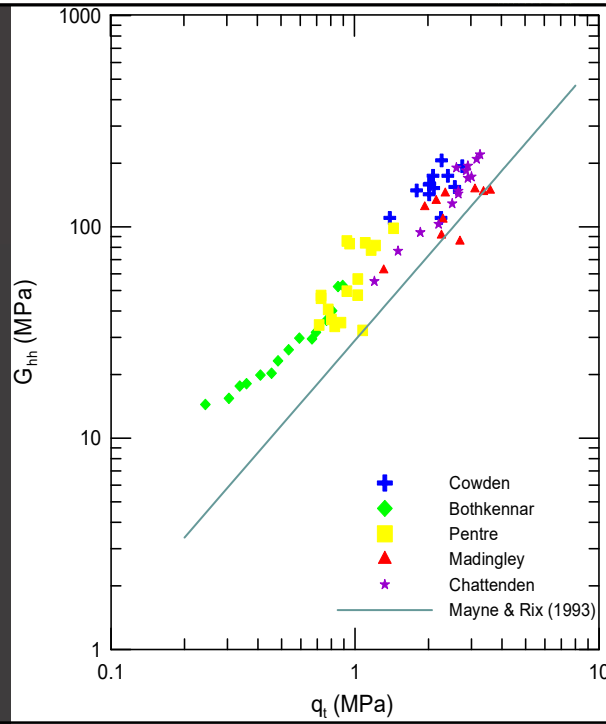
IMP



G_{HH} in UK soils

what you correlate with

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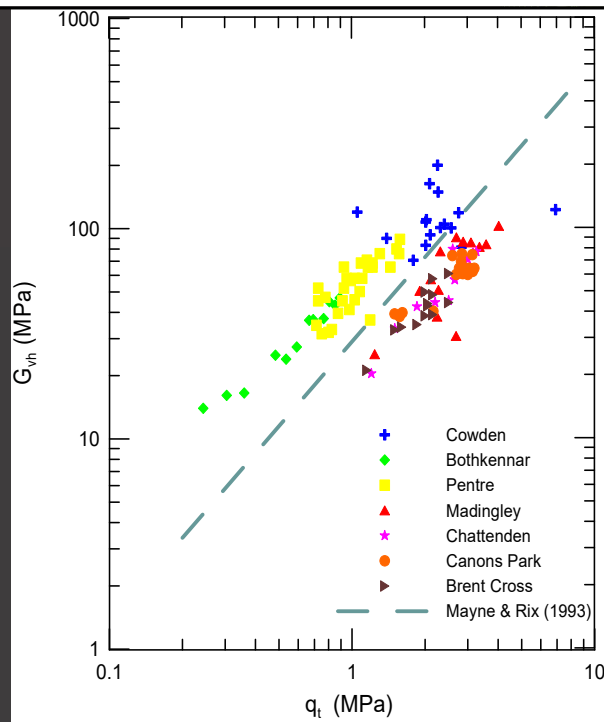
An UPDATE!



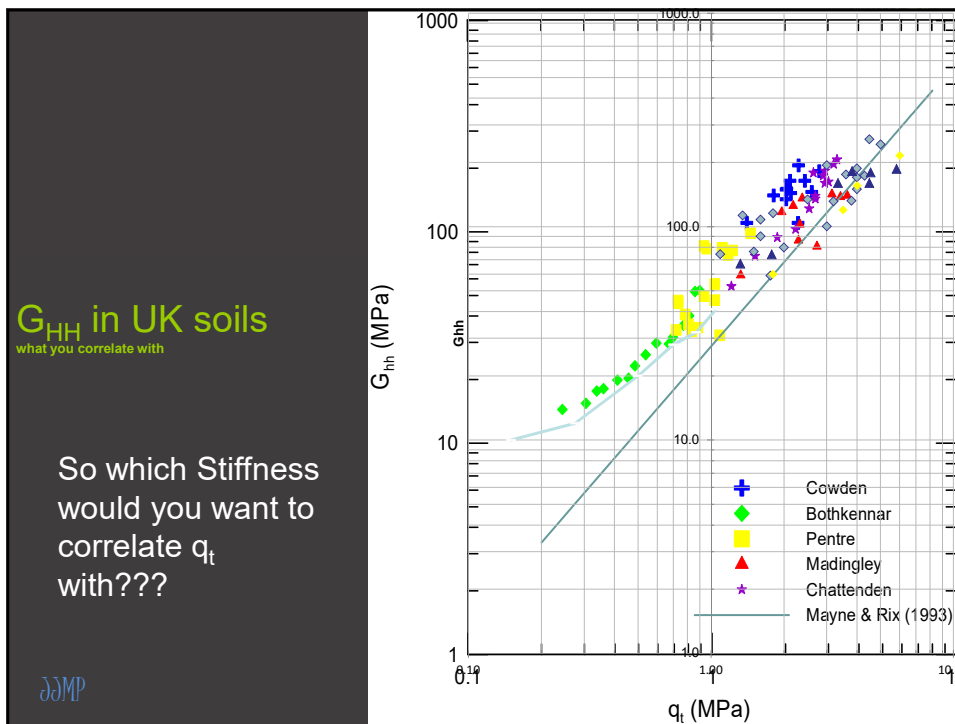
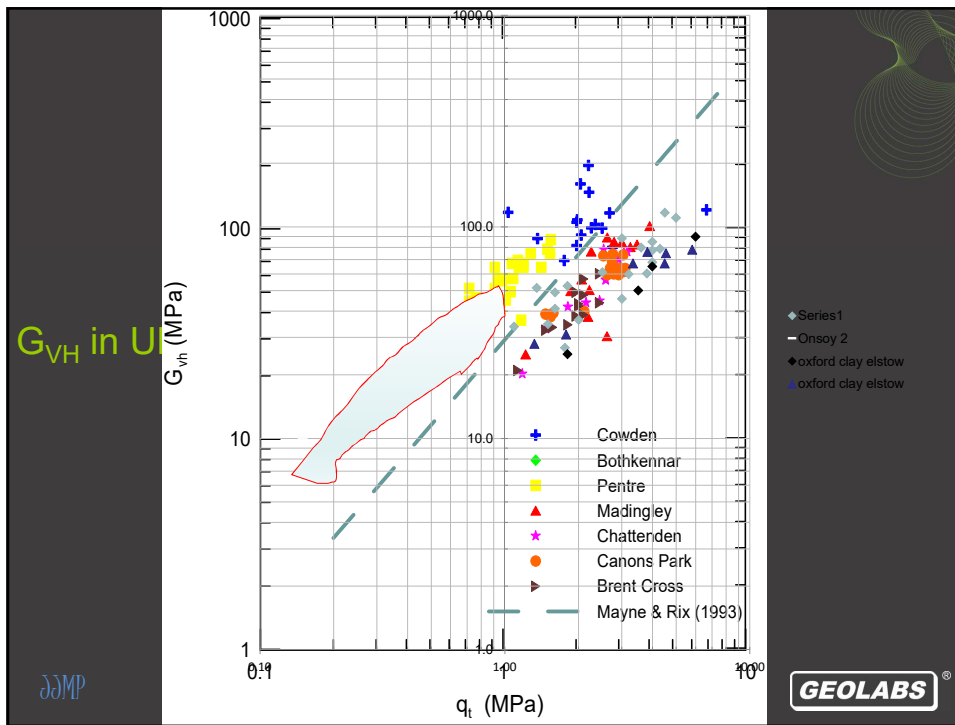
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G_{VH} in UK soils



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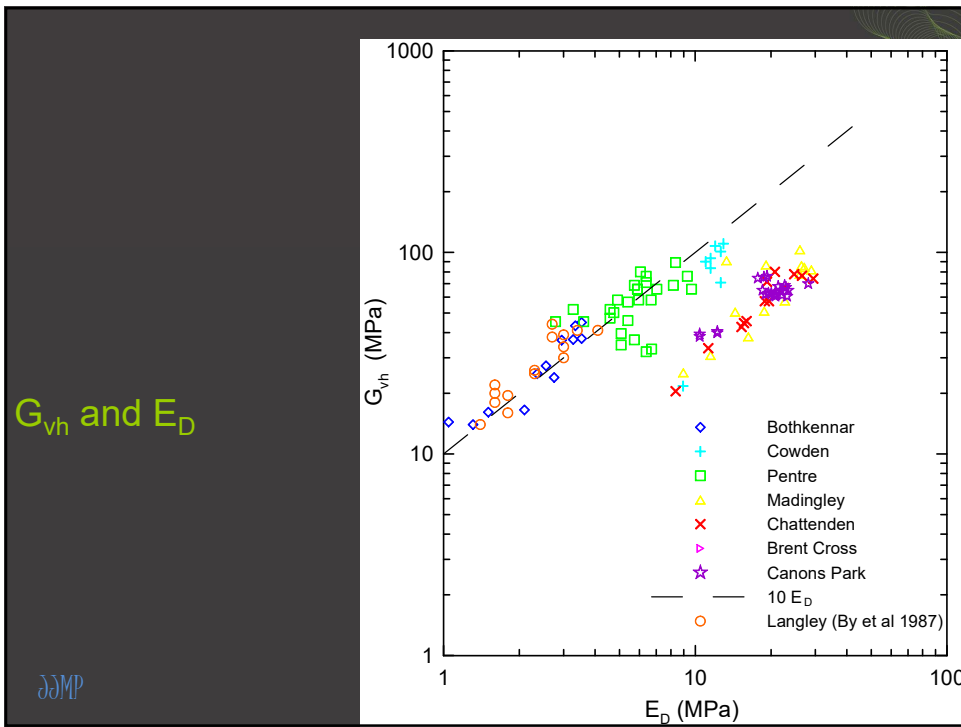


And another CONSIDERATION



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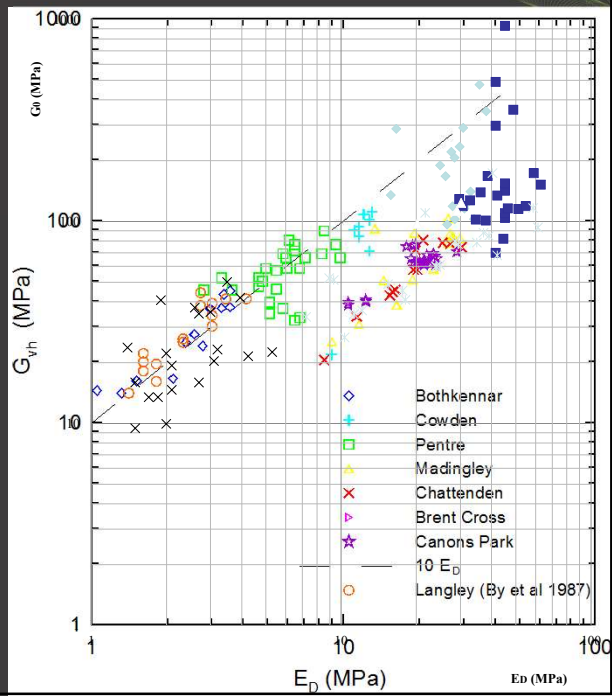
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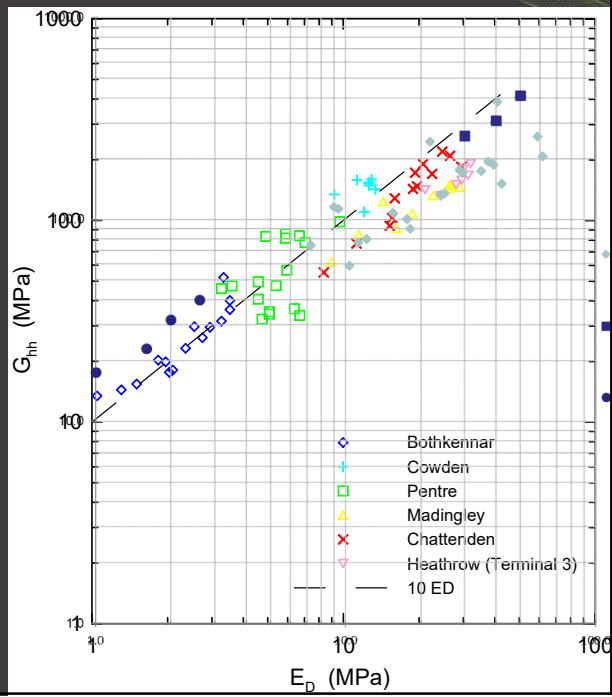
G_{vh} and E_D

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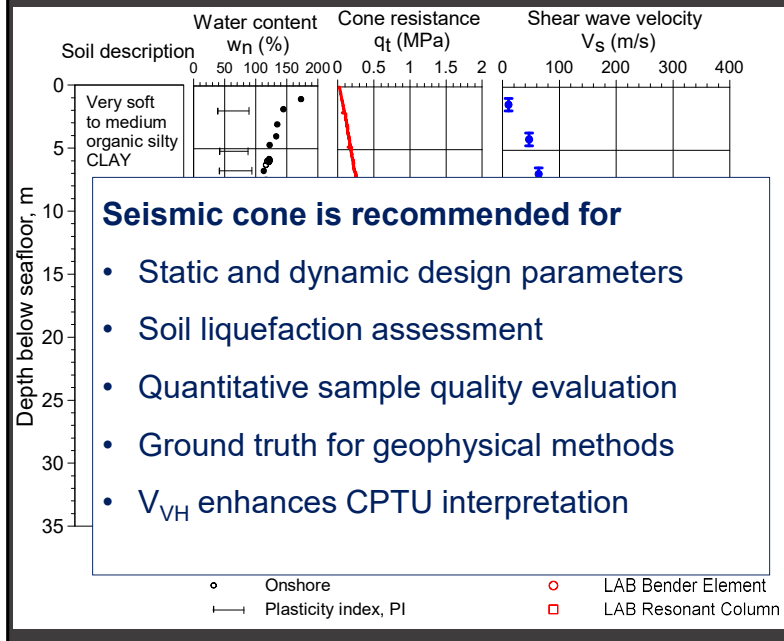
G_{hh} and E_D

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CASPIAN SEA SOFT CLAY

Slide 68



What with what

- q_t seems to link best with G_{hh}
- q_t seems to link best with E_D
- E_D seems to link best with G_{hh}

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Seismic Measurements

- Martin Fahey (2001) said that the addition of seismic measurements to the CPTU should become the next standard/routine form of the CPTU.
- We have come a long way and I think this is now proving to be the case
- Certainly the addition of geophones to the CPT enables downhole seismic testing to be undertaken in a very cost effective way, but remember that it is V_{vh} that is being measured
- If it is that q_t correlates with G_{hh} then, as we are actually deriving G_{vh} from the seismic cone, is there potential here for assessment of stiffness anisotropy?? Needs much more work!

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Cone Pressuremeter

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Cone Pressuremeter (CPM)

Cone Pressuremeter (CPM) = Pressuremeter module mounted behind a standard electrical cone penetrometer.

Advantages over conventional pressuremeters:

1. Uses standard CPT rigs
2. Operator independent, thus very repeatable
3. Clear ID of soils to be tested via CPT data
4. Know where in soil profile you are based on results of CPTU
5. Can combine with results of CPTU at same depth and same location

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CPM – Mechanical Details

The Pressuremeter module: 43.7 mm diameter, L/D = 10, attached behind 15 m² CPT or CPTU.

The Pressuremeter cell = cylindrical rubber membrane inflated by nitrogen gas. Membrane is protected during insertion by an additional steel reinforced rubber membrane

Measurements of inflation pressure and cavity strain are recorded at mid-height of the module by instrumentation at three locations, 120° apart. The maximum radial strain is 50%.

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Cone Pressuremeter

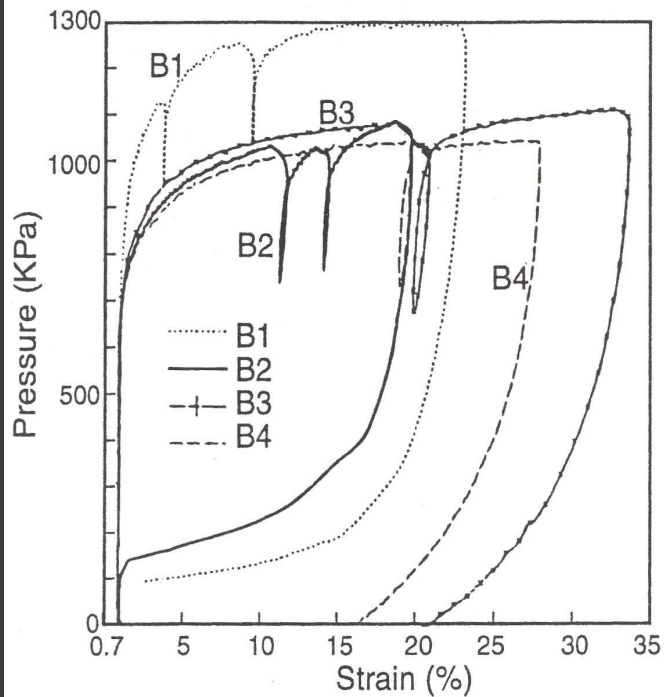


Tests in Clays

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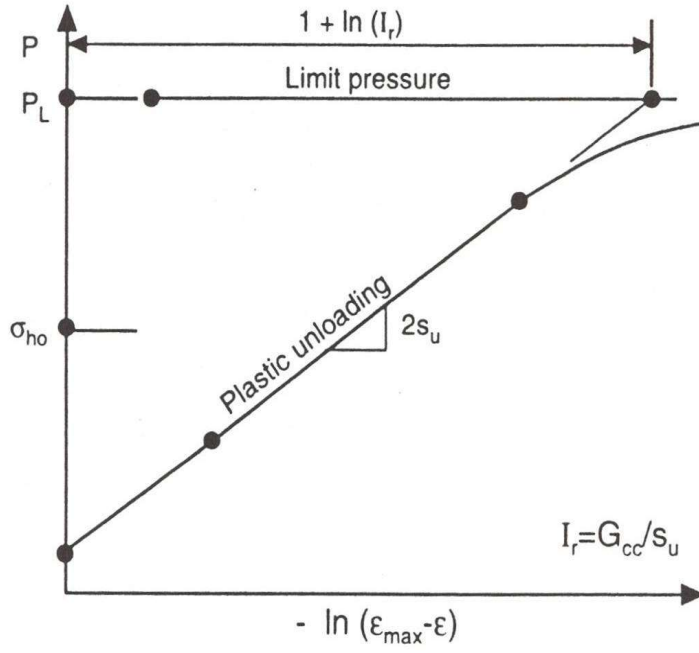
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Typical CPM tests with unload reload loops



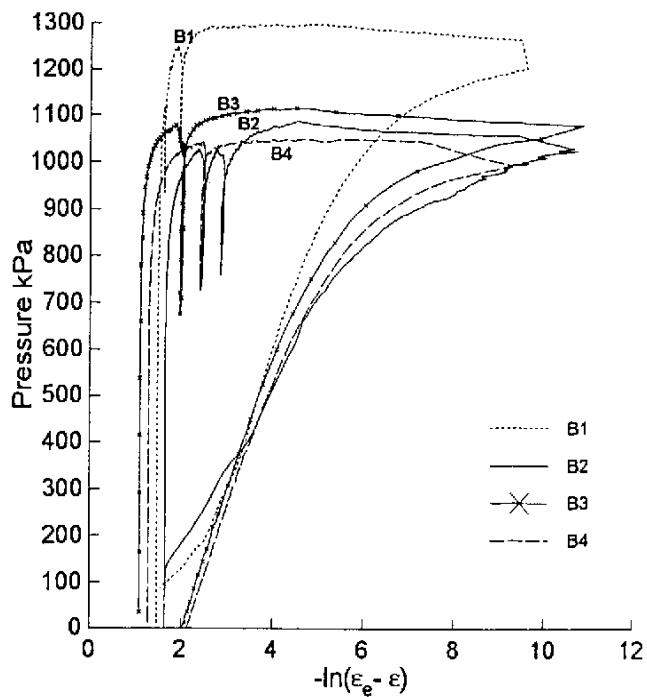
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Theory
(Houlsby
and Withers
1988)

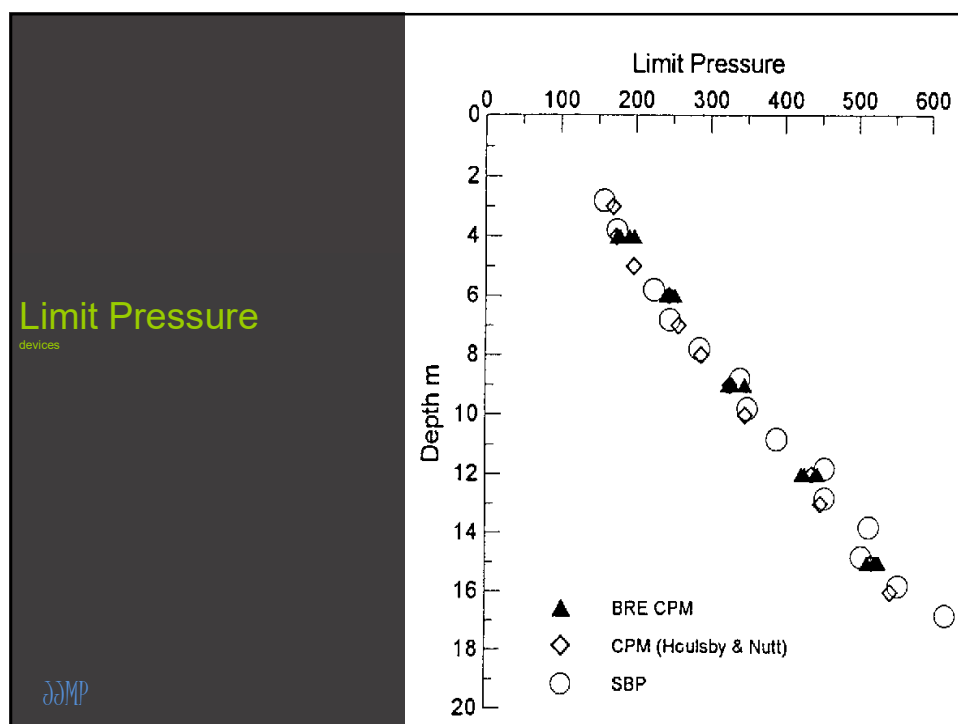
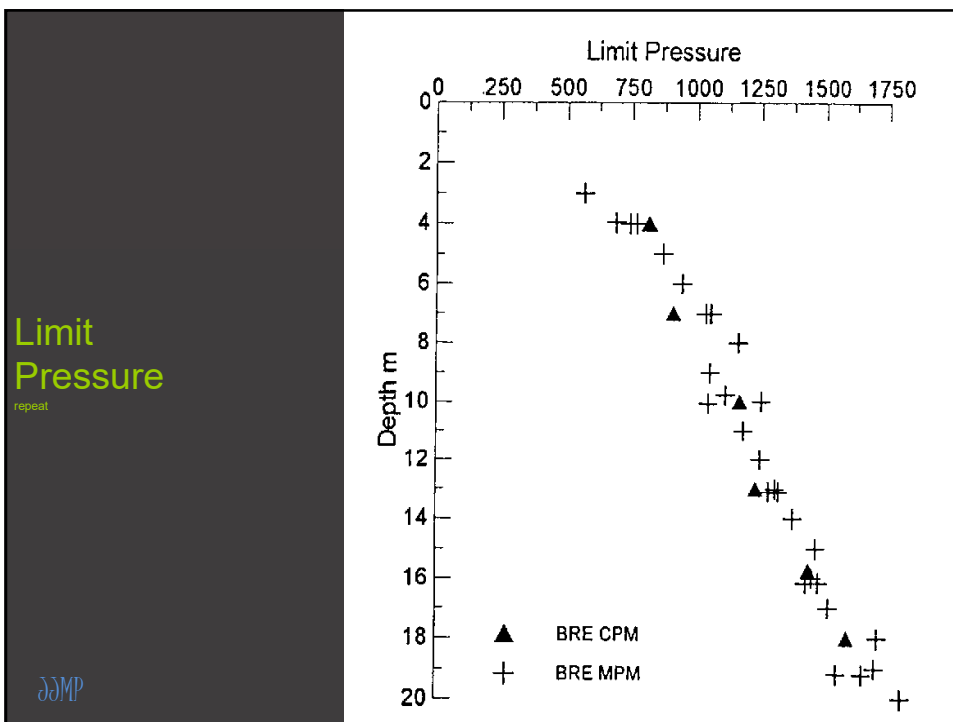


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Interpretation



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CPM shear strength soft clay

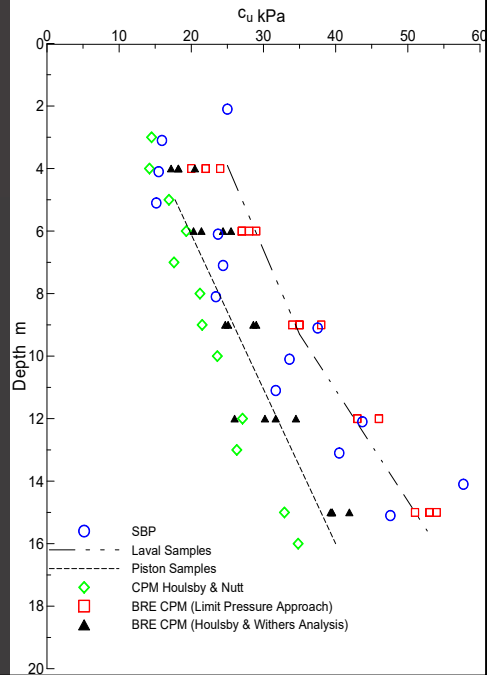
Bothkennar, UK

Undrained shear strength from CPM, SBP and laboratory triaxial tests

There are variations in s_u from different interpretation methods – continued topic of research

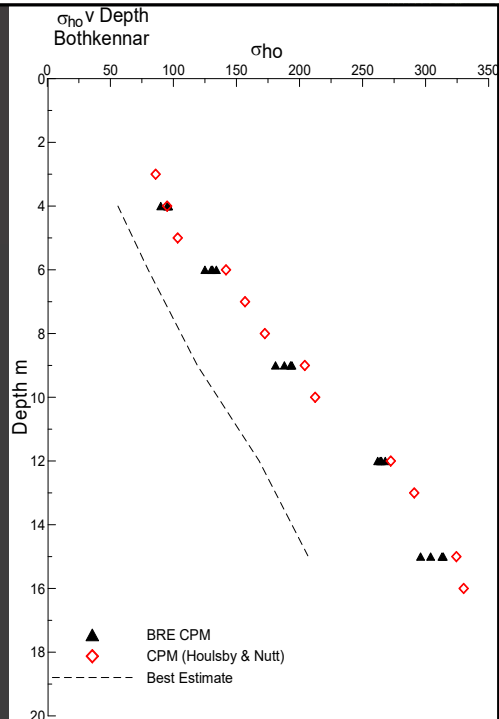
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Estimates of c_u for the Bothkennar Site obtained from in situ and laboratory tests

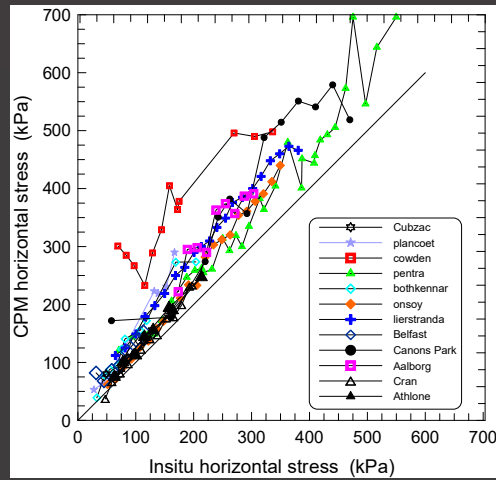


Bothkennar - σ_{ho}

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σ_{ho} vs best estimate all sites



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Yu correction (for length to diameter 10:1)

$$\sigma_{hoc} = \sigma_{hohw} - c_u [0.63 + 0.0733 \ln(I_r)]$$

- σ_{hoc} is the corrected horizontal stress
- σ_{hohw} is the Houlsby and Withers derived value
- c_u is the strength of the soil
- I_r is the rigidity index for the soil

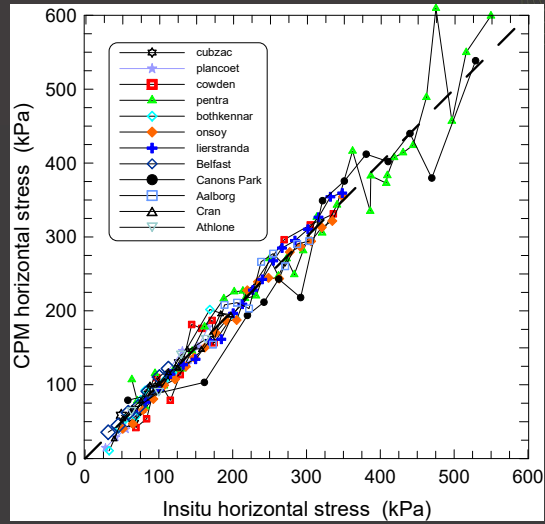
Basically c_u related correction

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σ_{hoymod} vs best estimate all sites

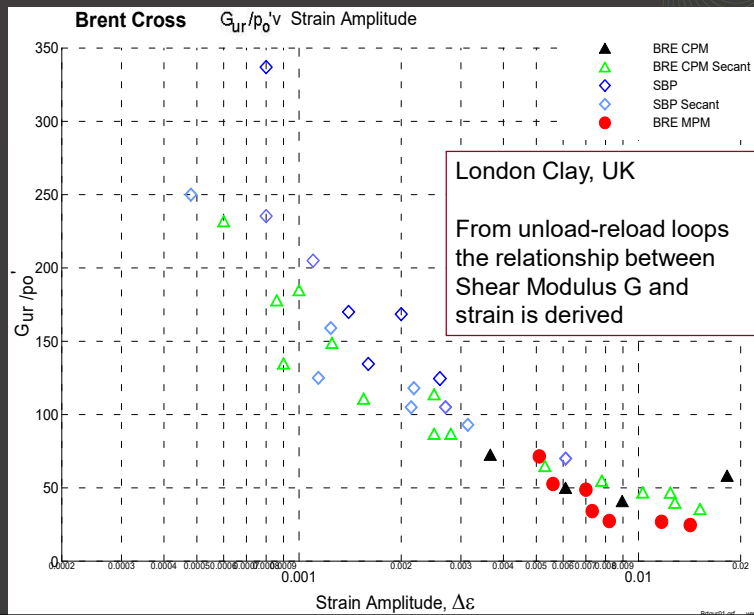
Using the modified Yu correction



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London Clay typical



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Bothkennar stiffness non typical

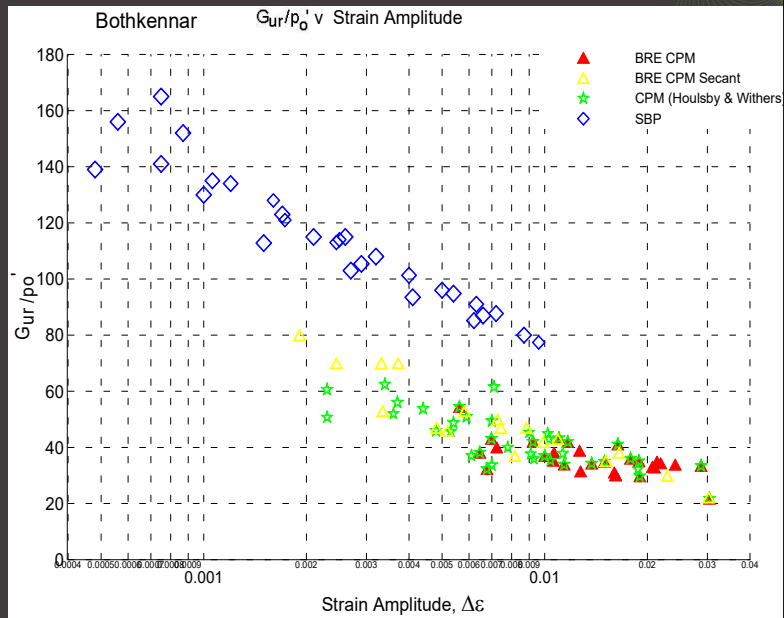


Figure 7.4.24

Cone pressuremeter - can give us

Clays:

- assessment/measurement of shear strength
- stress/ strain and hence stiffness with strain
- potential for assessment of horizontal stress – much improved

Sands:

clean sands

- initial state parameter needs q_c
- relative density needs q_c
- **stiffness**
- friction angle may need q_c
- horizontal stress needs q_c

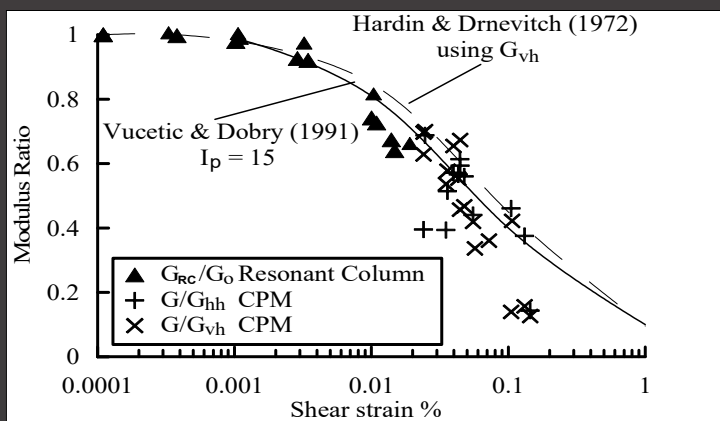
Is there potential for better combined assessments?

Linking the two

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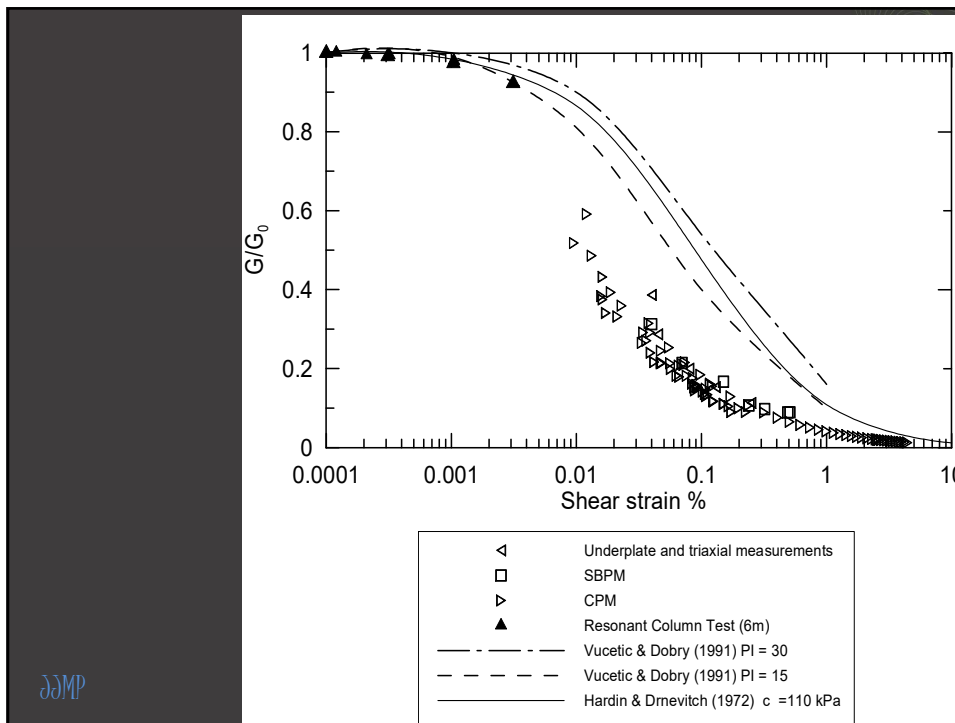
Non linear models of shear modulus and shear strain



Resonant column and CPM data from Pentre site

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Seismic and CPM Measurements

- Developments in procedures to extrapolate from small strain stiffness to larger strains will make the use of both tools increasingly powerful.

Summary – Additional CPTU Sensors

1. Seismic CPTU – well proven technology, becoming increasingly popular. But BE WARE
2. Cone Pressuremeter – limited availability, research in progress on interpretation procedures. Greatest potential is for estimating K_0 and shear stress-strain degradation curve. But there is also potential for better combined parameter assessments in sands?

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Conclusions and finally

- Combination of tests can be most powerful and give much better characterisation of soils and sites,
- So if we use a Seismic Cone Pressuremeter we have everything and redundancy!!
- Life is wonderful

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Thank you ?

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